...eine starke Verbindung

YDEEVNEDEKLARATION

DoP nr.: **MKT-342**- da

- **Varetypens unikke identifikationskode:** **Injektionssystem VMH i beton**
- **Tilsigtet anvendelse:** Limede ankre til forankring i beton, se bilag / Annex B
- **Fabrikant:** MKT Metall-Kunststoff-Technik GmbH & Co.KG
  Auf dem Immel 2
  67685 Weilerbach
- **System eller systemer til vurdering og kontrol af konstansen af ydeevnen:** 1
- **Europæisk vurderingsdokument:** **EAD 330499-01-0601**
  Europæisk teknisk vurdering: **ETA-17/0716, 22.11.2019**
  Teknisk vurderingsorgan: DIBt, Berlin
  Notificeret organ/notificerede organer: NB 1343 – MPA, Darmstadt
- **Deklareret ydeevne/deklarerede ydeevner:**

Væsentlige funktioner	Ydeevne
Mekanisk modstandsdygtighed og stabilitet (BWR 1)	
Karakteristiske modstande under trækbelastning (statiske og kvasistatiske effekter)	Bilag / Annex C1, C3, C6, C8
Karakteristiske modstande under tværgående stress (statiske og kvasistatiske effekter)	Bilag / Annex C2, C4, C7, C9
Forskydninger (statiske og kvasistatiske effekter)	Bilag / Annex C11 – C13
Karakteristisk modstand og forskydninger for seismisk ydeevne kategori C1 + C2	Bilag / Annex C5, C10, C11
Holdbarhed	Bilag / Annex B1
Hygiejne, sundhed og miljø (BWR 3)	
Indhold, emission og / eller frigivelse af farlige stoffer	Ydeevne ikke bedømt

Ydeevnen for den vare, der er anført ovenfor, er i overensstemmelse med den deklarerede ydeevne. Denne ydeevnedeklaration er udarbejdet i overensstemmelse med forordning (EU) nr. 305/2011 på eneansvar af den fabrikant, der er anført ovenfor.

Underskrevet for fabrikanten og på dennes vegne af:

Stefan Weustenhagen
(CEO)
Weilerbach, 22.11.2019

p.p.
Dipl.-Ing. Detlef Bigalke
(Leder af produktudvikling)

Originalen af denne erklæringserklæring blev skrevet på tysk. I tilfælde af afvigelser i oversættelsen er den tyske udgave gyldig.

Specification of intended use

Injection System VMH		Threaded rod	Internally threaded anchor rod	Rebar
Static or quasi-static action		M8 - M30 zinc plated, A2, A4, HCR	VMU-IG M6 - VMU-IG M20 zinc plated[1], A4, HCR	Ø8 - Ø32
Seismic action	performance category C1	M8 - M30 zinc plated[1], A4, HCR	-	Ø8 - Ø32
	performance category C2	M12 – M24 zinc plated[1] (property class 8.8) A4, HCR (poperty class ≥ 70)	-	-
Base materials		compacted, reinforced or unreinforced normal weight concrete (without fibers) acc. to EN 206:2013		
		strength classes acc. to EN 206:2013: C20/25 to C50/60		
		cracked or uncracked concrete		
Temperature Range I	-40 °C to +80 °C	max. long term temperature +50 °C and max. short term temperature +80 °C		
Temperature Range II	-40 °C to +120 °C	max. long term temperature +72 °C and max. short term temperature +120 °C		
Temperature Range III	-40 °C to +160 °C	max. long term temperature +100 °C and max. short term temperature +160 °C		

[1] except hot-dip galvanised

Use conditions (Environmental conditions):

- Structures subject to dry internal conditions (all materials).
- For all other conditions according to EN 1993-1-4:2006+A1:2015 corresponding to corrosion resistance classes:
  - Stainless steel A2 according to Annex A4, Table A1: CRC II
  - Stainless steel A4 according to Annex A4, Table A1: CRC III
  - High corrosion resistant steel HCR according to Annex A4, Table A1: CRC V

Design:

- Verifiable calculation notes and drawings are prepared taking account of the loads to be anchored. The position of the anchor is indicated on the design drawings (e. g. position of the anchor relative to reinforcement or to supports, etc.)
- Anchorages are designed under the responsibility of an engineer experienced in anchorages and concrete work
- Anchorages are designed in accordance with EN 1992-4:2018 or Technical Report TR 055

Installation:

- Dry or wet concrete or waterfilled drill holes (not seawater)
- Hole drilling by hammer or compressed air drill or vacuum drill mode
- Overhead installation allowed
- Anchor installation carried out by appropriately qualified personnel and under the responsibility of the person competent for technical matters on site
- Internally threaded anchor rod: Screws and threaded rods (incl. nut and washer) must at least correspond to the material and strength class of the internally threaded anchor rod used

Table B1: Installation parameters for threaded rods

Threaded rod				M8	M10	M12	M16	M20	M24	M27	M30
Diameter of threaded rod		$d=d_{nom}$	[mm]	8	10	12	16	20	24	27	30
Nominal drill hole diameter		d_0	[mm]	10	12	14	18	22	28	30	35
Effective anchorage depth		$h_{ef,min}$	[mm]	60	60	70	80	90	96	108	120
		$h_{ef,max}$	[mm]	160	200	240	320	400	480	540	600
Diameter of clearance hole in the fixture[2]	Pre-setting installation	$d_f \leq$	[mm]	9	12	14	18	22	26	30	33
	Through setting installation	$d_f \leq$	[mm]	12	14	16	20	24	30	33	40
Installation torque		$T_{inst} \leq$	[Nm]	10	20	40 (35)[1]	60	100	170	250	300
Minimum thickness of member		h_{min}	[mm]	h_{ef} + 30 mm ≥ 100 mm			$h_{ef} + 2d_0$				
Minimum spacing		s_{min}	[mm]	40	50	60	75	95	115	125	140
Minimum edge distance		c_{min}	[mm]	35	40	45	50	60	65	75	80

[1] Installation torque for M12 with steel grade 4.6

[2] For applications under seismic loading the diameter of clearance hole in the fixture shall be at maximum d_{nom} + 1mm or alternatively the annular gap between fixture and threaded rod shall be completely filled with mortar

Table B2: Installation parameters for internally threaded anchor rods

Internally threaded anchor rod			IG-M 6	IG-M 8	IG-M 10	IG-M 12	IG-M 16	IG-M 20
Inner diameter of threaded rod	d_2	[mm]	6	8	10	12	16	20
Outer diameter of threaded rod[1]	$d=d_{nom}$	[mm]	10	12	16	20	24	30
Nominal drill hole diameter	d_0	[mm]	12	14	18	22	28	35
Effective anchorage depth	$h_{ef,min}$	[mm]	60	70	80	90	96	120
	$h_{ef,max}$	[mm]	200	240	320	400	480	600
Diameter of clearance hole in the fixture	$d_f \leq$	[mm]	7	9	12	14	18	22
Installation torque	$T_{inst} \leq$	[Nm]	10	10	20	40	60	100
Minimum screw-in depth	l_{IG}	[mm]	8	8	10	12	16	20
Minimum thickness of member	h_{min}	[mm]	h_{ef} + 30 mm ≥ 100 mm		$h_{ef} + 2d_0$			
Minimum spacing	s_{min}	[mm]	50	60	75	95	115	140
Minimum edge distance	c_{min}	[mm]	40	45	50	60	65	80

[1] With metric thread acc. to EN 1993-1-8:2005+AC:2009

Table B3: Installation parameters for rebar

Rebar			Ø 8	Ø 10	Ø 12	Ø 14	Ø 16	Ø 20	Ø 24	Ø 25	Ø 28	Ø 32
Diameter of rebar	$d=d_{nom}$	[mm]	8	10	12	14	16	20	24	25	28	32
Nominal drill hole diameter[1]	d_0	[mm]	10 | 12	12 | 14	14 | 16	18	20	25	32	32	35	40
Effective anchorage depth	$h_{ef,min}$	[mm]	60	60	70	75	80	90	96	100	112	128
	$h_{ef,max}$	[mm]	160	200	240	280	320	400	480	500	560	640
Minimum thickness of member	h_{min}	[mm]	h_{ef} + 30 mm ≥ 100 mm		$h_{ef} + 2d_0$							
Minimum spacing	s_{min}	[mm]	40	50	60	70	75	95	120	120	130	150
Minimum edge distance	c_{min}	[mm]	35	40	45	50	50	60	70	70	75	85

[1] For diameter ∅8, ∅10 and ∅12 both nominal drill hole diameter can be used

Table B4: Parameter cleaning and setting tools

Threaded rod	Internally threaded anchor rod 	Rebar 	Drill bit Ø 	Brush Ø 	min. Brush Ø
[-]	[-]	Ø [mm]	d_0 [mm]	d_b [mm]	$d_{b,min}$ [mm]
M8		8	10	11,5	10,5
M10	VMU-IG M 6	8 / 10	12	13,5	12,5
M12	VMU-IG M 8	10 / 12	14	15,5	14,5
		12	16	17,5	16,5
M16	VMU-IG M10	14	18	20,0	18,5
		16	20	22,0	20,5
M20	VMU-IG M12		22	24,0	22,5
		20	25	27,0	25,5
M24	VMU-IG M16		28	30,0	28,5
M27			30	31,8	30,5
		24/25	32	34,0	32,5
M30	VMU-IG M20	28	35	37,0	35,5
		32	40	43,5	40,5

Table B5: Retaining washer

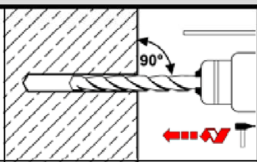
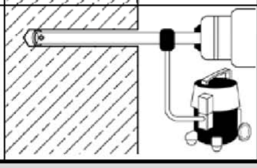
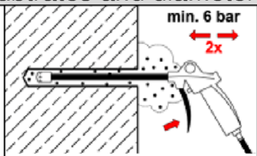
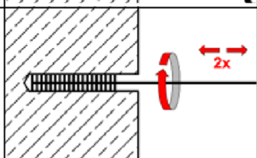
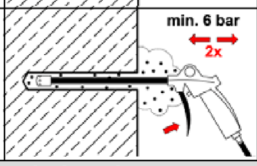
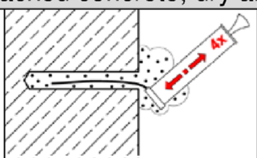
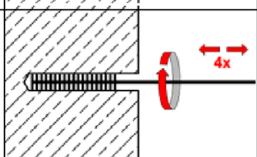
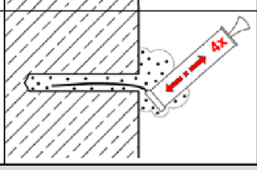
Drill bit Ø 		Installation direction and use		
d_0 [mm]	[-]	↓	→	↑
10	No **retaining washer** required			
12				
14				
16				
18	VM-IA 18	h_{ef} > 250mm	h_{ef} > 250mm	all
20	VM-IA 20			
22	VM-IA 22			
25	VM-IA 25			
28	VM-IA 28			
30	VM-IA 30			
32	VM-IA 32			
35	VM-IA 35			
40	VM-IA 40			

Vacuum drill bit
Drill bit diameter (d_0): all diameters
Vacuum drill bit (MKT Hollow drill bit SB, Würth Saugbohrer or Heller Duster Expert) and a class M vacuum with minimum negative pressure of 253 hPa and a flow rate of minimum 42 l/s

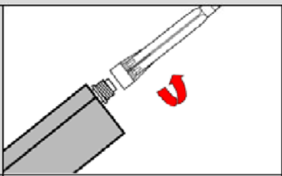
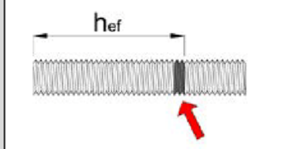
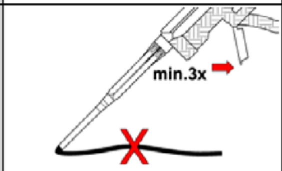
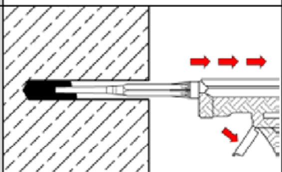
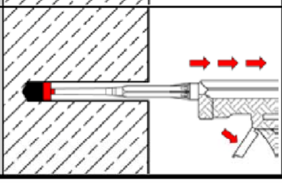
Recommended compressed air tool (min 6 bar)
Drill bit diameter (d_0): all diameters

Blow-out pump (volume 750ml)
Drill bit diameter (d_0): 10 mm to 20 mm
Drill hole depth (h_0): ≤ 10 d_{nom}
for uncracked concrete

Installation Instructions

Drilling of the hole			
1	1a		**Hammer drill oder compressed air drill** Drill with hammer drill or compressed air drill a hole into the base material to the size required by the selected anchor (Table B1, B2 or B3). Continue with <u>step 2.</u> In case of aborted drill hole, the drill hole shall be filled with mortar.
	1b		**Vacuum drill bit**: see Annex B3 Drill hole into the base material to the embedment size and embedment depth required by the selected anchor (table B1, B2 or B3). This drilling system removes dust and cleans the drill hole during drilling. Continue with <u>step 3</u>. In case of aborted hole, the drill hole shall be filled with mortar.
Cleaning (not applicable when using a vacuum drill)			
2	**Attention! Standing water in the drill hole must be removed before cleaning!**		
	<u>Cleaning with compressed air</u> all subtrates and diameters according to Annex B1		
	2a		Starting from the bottom or back of the drill hole, blow out the hole with compressed air (min. 6 bar) a minimum of **two** times until return air stream is free of noticeable dust. If the drill hole ground is not reached, an extension must be used.
	2b		Check brush diameter (Table B4). Brush the hole with an appropriate sized wire brush ≥ $d_{b,min}$ (Table B4) a minimum of **two** times. If the drill hole ground is not reached with the brush, an appropriate brush extension must be used.
	2c		Starting from the bottom or back of the drill hole, blow out the hole with compressed air (min. 6 bar) again a minimum of **two** times until return air stream is free of noticeable dust. If the drill hole ground is not reached, an extension must be used.
	<u>Manual cleaning</u> uncracked concrete, dry and wet drill holes; drill hole diameter d_0 ≤ 20mm and drill hole depth h_0 ≤ 10 d_{nom}		
	2a		Starting from the bottom or back of the drill hole, blow out the hole with the blow-out pump a minimum of **four** times.
	2b		Check brush diameter (Table B4). Brush the hole with an appropriate sized wire brush ≥ $d_{b,min}$ (Table B4) a minimum of **four** times. If the drill hole ground is not reached with the brush, an appropriate brush extension must be used.
	2c		Starting from the bottom or back of the drill hole blow out the hole again a minimum of **four** times.
	After cleaning, the drill hole has to be protected against re-contamination in an appropriate way, until dispensing the mortar in the drill hole. If necessary, the cleaning has to be repeated directly before dispensing the mortar. In-flowing water must not contaminate the drill hole again.		

Installation instructions (continuation)

Injection		
3		Attach the supplied static-mixing nozzle to the cartridge and load the cartridge into the correct dispensing tool. For every working interruption longer than the recommended working time (Table B6) as well as for new cartridges, a new static-mixer shall be used.
4	 	Prior to inserting the rod into the filled drill hole, the position of the embedment depth shall be marked on the threaded rod or rebar
5	 	Prior to dispensing into the drill hole, squeeze out separately a minimum of three full strokes and discard non-uniformly mixed adhesive components until the mortar shows a consistent grey colour.
6a		Starting from the bottom or back of the cleaned drill hole fill the hole up to approximately two-thirds with adhesive. Slowly withdraw the static mixing nozzle as the hole fills to avoid air pockets. If the drill hole ground is not reached, an appropriate extension nozzle shall be used. Observe working times given in Table B6.
6b		Retaining washer and mixer nozzle extensions shall be used according to Table B5 for the following applications: • Horizontal installation (horizontal direction) and ground installation (vertical downwards direction): Drill bit-Ø $d_0 \geq 18$ mm and anchorage depth $h_{ef} > 250$mm • Overhead installation: Drill bit-Ø $d_0 \geq 18$ mm

Installation instructions (continuation)

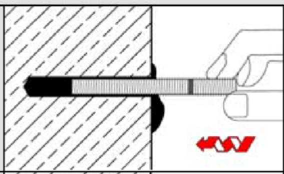
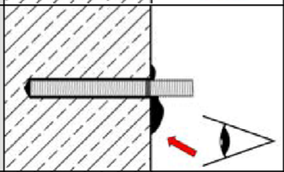
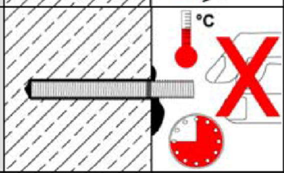
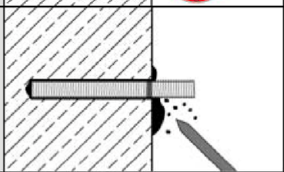
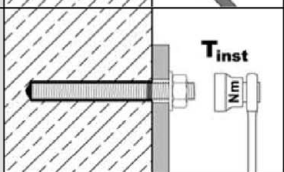
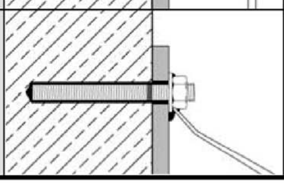
Inserting the anchor		
7		Push the fastening element into the hole while turning slightly to ensure proper distribution of the adhesive until the embedment depth is reached. The anchor shall be free of dirt, grease, oil or other foreign material.
8		After installation, the annular gap between anchor rod and concrete must be completely filled with mortar, in the case of push-through installation also in the fixture. If these requirements are not fulfilled, repeat application before end of working time! For overhead installation, the anchor should be fixed (e.g. by wedges).
9		Allow the adhesive to cure to the specified time prior to applying any load or torque. Do not move or load the anchor until it is fully cured (attend Table B6).
10		Remove excess mortar.
11		The fixture can be mounted after curing time. Apply installation torque T_{inst} according to Table B1 or B2.
12		In case of pre-setting installation, the annular gap between anchor rod and fixture may optionally be filled with mortar. Therefore, replace regular washer by washer with bore and plug on reducing adapter on static mixer. Annular gap is completely filled, when excess mortar seeps out.

Table B6: Working time and curing time

Concrete temperature	Working time	Minimum curing time	
		dry concrete	wet concrete
-5°C to -1°C	50 min	5 h	10 h
0°C to +4°C	25 min	3,5 h	7 h
+5°C to +9°C	15 min	2 h	4 h
+10°C to +14°C	10 min	1 h	2 h
+15°C to +19°C	6 min	40 min	80 min
+20°C to +29°C	3 min	30 min	60 min
+30°C to +40°C	2 min	30 min	60 min
Cartridge temperature	+ 5°C to + 40°C		

Table C1: Characteristic **steel resistance** for **threaded rods** under **tension load**

Threaded rod				**M8**	**M10**	**M12**	**M16**	**M20**	**M24**	**M27**	**M30**
Steel failure											
Cross sectional area		A_s	[mm²]	36,6	58,0	84,3	157	245	353	459	561
Characteristic resistance under tension load [1]											
Steel, zinc plated	Property class 4.6 and 4.8	$N_{Rk,s}$	[kN]	15 (13)	23 (21)	34	63	98	141	184	224
	Property class 5.6 and 5.8	$N_{Rk,s}$	[kN]	18 (17)	29 (27)	42	78	122	176	230	280
	Property class 8.8	$N_{Rk,s}$	[kN]	29 (27)	46 (43)	67	125	196	282	368	449
Stainless steel	A2, A4 and HCR Property class 50	$N_{Rk,s}$	[kN]	18	29	42	79	123	177	230	281
	A2, A4 and HCR Property class 70	$N_{Rk,s}$	[kN]	26	41	59	110	171	247	-	-
	A4 and HCR Property class 80	$N_{Rk,s}$	[kN]	29	46	67	126	196	282	-	-
Partial factor [2]											
Steel, zinc plated	Property class 4.6	$\gamma_{Ms,N}$	[-]	2,0							
	Property class 4.8	$\gamma_{Ms,N}$	[-]	1,5							
	Property class 5.6	$\gamma_{Ms,N}$	[-]	2,0							
	Property class 5.8	$\gamma_{Ms,N}$	[-]	1,5							
	Property class 8.8	$\gamma_{Ms,N}$	[-]	1,5							
Stainless steel	A2, A4 and HCR Property class 50	$\gamma_{Ms,N}$	[-]	2,86							
	A2, A4 and HCR Property class 70	$\gamma_{Ms,N}$	[-]	1,87						-	-
	A4 and HCR Property class 80	$\gamma_{Ms,N}$	[-]	1,6						-	-

[1] The characteristic resistances apply for all anchor rods with the cross sectional area A_s specified here: VMU-A, V-A, VM-A
For commercial standard threaded rods with a smaller cross sectional area (e.g. hot-dip galvanized threaded rods M8, M10 according to EN ISO 10684:2004 + AC:2009), the values in brackets are valid.

[2] In absence of other national regulations

Table C2: Characteristic **steel resistance** for **threaded rods** under shear load

Threaded rod				M 8	M 10	M 12	M 16	M 20	M 24	M 27	M 30
Steel failure											
Cross sectional area		A_s	[mm²]	36,5	58,0	84,3	157	245	353	459	561
Characteristic resistances under shear load[1]											
Steel failure without lever arm											
Steel, zinc plated	Property class 4.6 and 4.8	$V^0_{Rk,s}$	[kN]	9 (8)	14 (13)	20	38	59	85	110	135
	Property class 5.6 and 5.8	$V^0_{Rk,s}$	[kN]	11 (10)	17 (16)	25	47	74	106	138	168
	Property class 8.8	$V^0_{Rk,s}$	[kN]	15 (13)	23 (21)	34	63	98	141	184	224
Stainless steel	A2, A4 and HCR, property class 50	$V^0_{Rk,s}$	[kN]	9	15	21	39	61	88	115	140
	A2, A4 and HCR, property class 70	$V^0_{Rk,s}$	[kN]	13	20	30	55	86	124	-	-
	A4 and HCR, property class 80	$V^0_{Rk,s}$	[kN]	15	23	34	63	98	141	-	-
Steel failure with lever arm											
Steel, zinc plated	Property class 4.6 and 4.8	$M^0_{Rk,s}$	[Nm]	15 (13)	30 (27)	52	133	260	449	666	900
	Property class 5.6 and 5.8	$M^0_{Rk,s}$	[Nm]	19 (16)	37 (33)	65	166	324	560	833	1123
	Property class 8.8	$M^0_{Rk,s}$	[Nm]	30 (26)	60 (53)	105	266	519	896	1333	1797
Stainless steel	A2, A4 and HCR, property class 50	$M^0_{Rk,s}$	[Nm]	19	37	66	167	325	561	832	1125
	A2, A4 and HCR, property class 70	$M^0_{Rk,s}$	[Nm]	26	52	92	232	454	784	-	-
	A4 and HCR, property class 80	$M^0_{Rk,s}$	[Nm]	30	59	105	266	519	896	-	-
Partial factor[2]											
Steel, zinc plated	Property class 4.6	$\gamma_{Ms,V}$	[-]	1,67							
	Property class 4.8	$\gamma_{Ms,V}$	[-]	1,25							
	Property class 5.6	$\gamma_{Ms,V}$	[-]	1,67							
	Property class 5.8	$\gamma_{Ms,V}$	[-]	1,25							
	Property class 8.8	$\gamma_{Ms,V}$	[-]	1,25							
Stainless steel	A2, A4 and HCR, property class 50	$\gamma_{Ms,V}$	[-]	2,38							
	A2, A4 and HCR, property class 70	$\gamma_{Ms,V}$	[-]	1,56						-	-
	A4 and HCR, property class 80	$\gamma_{Ms,V}$	[-]	1,33						-	-

[1] The characteristic resistances apply for all anchor rods with the cross sectional area A_s specified here: VMU-A, V-A, VM-A
For commercial standard threaded rods with a smaller cross sectional area (e.g. hot-dip galvanized threaded rods M8, M10 according to EN ISO 10684:2004 + AC:2009), the values in brackets are valid

[2] In absence of other national regulations

Table C3: Characteristic values of **tension loads** for **threaded rods** under static or quasi-static action

Threaded rod			**M8**	**M10**	**M12**	**M16**	**M20**	**M24**	**M27**	**M30**
Steel failure										
Characteristic resistance	$N_{Rk,s}$	[kN]	$A_s \cdot f_{uk}$ or see Table C1							
Partial factor	$\gamma_{Ms,N}$	[-]	see Table C1							
Combined pull-out and concrete failure										
Characteristic bond resistance in uncracked concrete C20/25										
Temperature range I: 80°C / 50°C	$\tau_{Rk,ucr}$	[N/mm²]	17	17	16	15	14	13	13	13
Temperature range II: 120°C / 72°C	$\tau_{Rk,ucr}$	[N/mm²]	15	14	14	13	12	12	11	11
Temperature range III: 160°C / 100°C	$\tau_{Rk,ucr}$	[N/mm²]	12	11	11	10	9,5	9,0	9,0	9,0
Characteristic bond resistance in cracked concrete C20/25										
Temperature range I: 80°C / 50°C	$\tau_{Rk,cr}$	[N/mm²]	7,0	7,5	8,0	9,0	8,5	7,0	7,0	7,0
Temperature range II: 120°C / 72°C	$\tau_{Rk,cr}$	[N/mm²]	6,0	6,5	7,0	7,5	7,0	6,0	6,0	6,0
Temperature range III: 160°C / 100°C	$\tau_{Rk,cr}$	[N/mm²]	5,5	5,5	6,0	6,5	6,0	5,5	5,5	5,5
Reduction factor ψ^0_{sus} in concrete C20/25										
Temperature range I: 80°C / 50°C	ψ^0_{sus}	[-]	0,79							
Temperature range II: 120°C / 72°C	ψ^0_{sus}	[-]	0,75							
Temperature range III: 160°C / 100°C	ψ^0_{sus}	[-]	0,66							
Increasing factors for concrete	ψ_c	C25/30	1,02							
		C30/37	1,04							
		C35/45	1,07							
		C40/50	1,08							
		C45/55	1,09							
		C50/60	1,10							
Concrete cone failure										
Factor k_1 — uncracked concrete	$k_{ucr,N}$	[-]	11,0							
Factor k_1 — cracked concrete	$k_{cr,N}$	[-]	7,7							
Splitting failure										
Edge distance — $h/h_{ef} \geq 2,0$	$c_{cr,sp}$	[mm]	$1,0\ h_{ef}$							
Edge distance — $2,0 > h/h_{ef} > 1,3$	$c_{cr,sp}$	[mm]	$2 \cdot h_{ef}\ (2,5 - h / h_{ef})$							
Edge distance — $h/h_{ef} \leq 1,3$	$c_{cr,sp}$	[mm]	$2,4\ h_{ef}$							
Spacing	$s_{cr,sp}$	[mm]	$2\ c_{cr,sp}$							
Installation factor										
dry or wet concrete — Vacuum cleaning	γ_{inst}	[-]	1,2							
dry or wet concrete — Manual cleaning	γ_{inst}	[-]	1,2				NPA			
dry or wet concrete — Compressed air cleaning	γ_{inst}	[-]	1,0							
water filled drill hole — Compressed air cleaning	γ_{inst}	[-]	1,4							

Table C4: Characteristic values of **shear loads** for **threaded rods** under static or quasi-static action

Threaded rod			M8	M10	M12	M16	M20	M24	M27	M30
Steel failure without lever arm										
Characteristic resistance Steel, zinc plated Class 4.6, 4.8, 5.6 and 5.8	$V^0_{Rk,s}$	[kN]	$0{,}6 \cdot A_s \cdot f_{uk}$ or see Table C2							
Characteristic resistance Steel, zinc plated, class 8.8, stainless steel A2, A4 and HCR	$V^0_{Rk,s}$	[kN]	$0{,}5 \cdot A_s \cdot f_{uk}$ or see Table C2							
Ductility factor	k_7	[-]	1,0							
Partial factor	$\gamma_{Ms,V}$	[-]	see Table C2							
Steel failure with lever arm										
Characteristic bending resistance	$M^0_{Rk,s}$	[Nm]	$1{,}2 \cdot W_{el} \cdot f_{uk}$ or see Table C2							
Elastic section modulus	W_{el}	[mm³]	31	62	109	277	541	935	1387	1874
Partial factor	$\gamma_{Ms,V}$	[-]	see Table C2							
Concrete pry-out failure										
Pry-out factor	k_8	[-]	2,0							
Concrete edge failure										
Effective length of anchor	l_f	[mm]	min (h_{ef};12 d_{nom})						min (h_{ef};300mm)	
Outside diameter of anchor	d_{nom}	[mm]	8	10	12	16	20	24	27	30
Installation factor	γ_{inst}	[-]	1,0							

Table C5: Characteristic values of **tesion loads** for **threaded rods** under **seismic action**, performance category **C1 + C2** [1)]

Threaded rod				M8	M10	M12	M16	M20	M24	M27	M30
Steel failure											
Characteristic resistance		$N_{Rk,s,eq,C1}$	[kN]	1,0 • $N_{Rk,s}$							
		$N_{Rk,s,eq,C2}$	[kN]	NPA		1,0 • $N_{Rk,s}$				NPA	
Partial factor		$\gamma_{Ms,N}$	[-]	siehe Tabelle C1							
Combined pull-out and concrete failure											
Characteristic bond resistance											
Temperatur-range	I: 80°C / 50°C	$\tau_{Rk,eq,C1}$	[N/mm²]	7,0	7,5	8,0	9,0	8,5	7,0	7,0	7,0
		$\tau_{Rk,eq,C2}$	[N/mm²]	NPA		3,6	3,5	3,3	2,3	NPA	
	II: 120°C / 72°C	$\tau_{Rk,eq,C1}$	[N/mm²]	6,0	6,5	7,0	7,5	7,0	6,0	6,0	6,0
		$\tau_{Rk,eq,C2}$	[N/mm²]	NPA		3,1	3,0	2,8	2,0	NPA	
	III: 160°C / 100°C	$\tau_{Rk,eq,C1}$	[N/mm²]	5,5	5,5	6,0	6,5	6,0	5,5	5,5	5,5
		$\tau_{Rk,eq,C2}$	[N/mm²]	NPA		2,5	2,7	2,5	1,8	NPA	
Installation factor											
Compressed air cleaning	dry or wet concrete	γ_{inst}	[-]	1,0							
	water filled drill hole	γ_{inst}	[-]	1,4							
Vacuum cleaning	dry or wet concrete	γ_{inst}	[-]	1,2							

[1)] Materials and property classes according to Annex B1

Table C6: Characteristic values of **tesion loads** for **threaded rods** under **seismic action**, performance category **C1 + C2** [1)]

Threaded rod				M8	M10	M12	M16	M20	M24	M27	M30
Steel failure <u>without</u> lever arm											
Characteristic resistance		$V_{Rk,s,eq,C1}$	[kN]	0,7 • $V^0_{Rk,s}$							
		$V_{Rk,s,eq,C2}$	[kN]	NPA		0,7 • $V^0_{Rk,s}$				NPA	
Partial factor		$\gamma_{Ms,N}$	[-]	siehe Tabelle C2							
Steel failure <u>without</u> lever arm											
Charactersitic bending resistance		$M^0_{Rk,s,eq}$	[Nm]	No Performance Assessed (NPA)							
Installation factor		γ_{inst}	[-]	1,0							
Factor for anchorages	without hole clearance	α_{gap}	[-]	1,0							
	with hole clearance between fastener and fixture	α_{gap}	[-]	0,5							

[1)] Materials and property classes according to Annex B1

Table C7: Characteristic values of **tension loads** for **internally threaded anchor rod** under static or quasi-static action

Internally threaded anchor rod				VMU-IG M 6	VMU-IG M 8	VMU-IG M 10	VMU-IG M 12	VMU-IG M 16	VMU-IG M 20
Steel failure [1]									
Characteristic resistance, steel, zinc plated, property class	5.8	$N_{Rk,s}$	[kN]	10	17	29	42	76	123
	8.8	$N_{Rk,s}$	[kN]	16	27	46	67	121	196
Partial factor		$\gamma_{Ms,N}$	[-]	1,5					
Characteristic resistance, stainless steel A4 / HCR, property class	70	$N_{Rk,s}$	[kN]	14	26	41	59	110	124 [2]
Partial factor		$\gamma_{Ms,N}$	[-]	1,87					2,86
Combined pull-out and concrete failure									
Characteristic bond resistance in uncracked concrete C20/25									
Temperature range	I: 80°C / 50°C	$\tau_{Rk,ucr}$	[N/mm²]	17	16	15	14	13	13
	II: 120°C / 72°C	$\tau_{Rk,ucr}$	[N/mm²]	14	14	13	12	12	11
	III: 160°C / 100°C	$\tau_{Rk,ucr}$	[N/mm²]	11	11	10	9,5	9,0	9,0
Characteristic bond resistance in cracked concrete C20/25									
Temperature range	I: 80°C / 50°C	$\tau_{Rk,cr}$	[N/mm²]	7,5	8,0	9,0	8,5	7,0	7,0
	II: 120°C / 72°C	$\tau_{Rk,cr}$	[N/mm²]	6,5	7,0	7,5	7,0	6,0	6,0
	III: 160°C / 100°C	$\tau_{Rk,cr}$	[N/mm²]	5,5	6,0	6,5	6,0	5,5	5,5
Reduction factor ψ^0_{sus} in concrete C20/25									
Temperature range	I: 80°C / 50°C	ψ^0_{sus}	[-]	0,79					
	II: 120°C / 72°C	ψ^0_{sus}	[-]	0,75					
	III: 160°C / 100°C	ψ^0_{sus}	[-]	0,66					
Increasing factors for concrete		ψ_c	C25/30	1,02					
			C30/37	1,04					
			C35/45	1,07					
			C40/50	1,08					
			C45/55	1,09					
			C50/60	1,10					
Concrete cone failure									
Factor k_1	uncracked concrete	$k_{ucr,N}$	[-]	11,0					
	cracked concrete	$k_{cr,N}$	[-]	7,7					
Splitting failure									
Edge distance	$h/h_{ef} \geq 2{,}0$	$c_{cr,sp}$	[mm]	$1{,}0\ h_{ef}$					
	$2{,}0 > h/h_{ef} > 1{,}3$			$2 \cdot h_{ef}\ (2{,}5 - h / h_{ef})$					
	$h/h_{ef} \leq 1{,}3$			$2{,}4\ h_{ef}$					
Spacing		$s_{cr,sp}$	[mm]	$2\ c_{cr,sp}$					
Installation factor									
dry or wet concrete	vaccum cleaning	γ_{inst}	[-]	1,2					
	manual cleaning	γ_{inst}	[-]	1,2			NPA		
	compressed air cleaning	γ_{inst}	[-]	1,0					
waterfilled drill hole		γ_{inst}	[-]	1,2					

[1] Fastening screws or threaded rods (incl. nut and washer) must comply with the appropriate material and property class of the internally threaded anchor rod. The characteristic tension resistance for steel failure of the given strength class are valid for the internally threaded anchor rod and the fastening element

[2] For VMU-IG M20: property class 50

Table C8: Characteristic values of **shear loads** for **internally threaded anchor rod** under static or quasi-static action

Internally threaded anchor rod					**VMU-IG M 6**	**VMU-IG M 8**	**VMU-IG M 10**	**VMU-IG M 12**	**VMU-IG M 16**	**VMU-IG M 20**
Steel failure without lever arm[1)]										
Steel, zinc plated	Characteristic resistance	property class 5.8	$V^0_{Rk,s}$	[kN]	5	9	15	21	38	61
	Characteristic resistance	property class 8.8	$V^0_{Rk,s}$	[kN]	8	14	23	34	60	98
	Partial factor		$\gamma_{Ms,V}$	[-]	1,25					
Stainless steel	Characteristic resistance A4 / HCR	property class 70	$V^0_{Rk,s}$	[kN]	7	13	20	30	55	62 [2)]
	Partial factor		$\gamma_{Ms,V}$	[-]	1,56					2,38
Ductility factor			k_7	[-]	1,0					
Steel failure with lever arm[1)]										
Steel, zinc plated	Characteristic bending resistance	property class 5.8	$M^0_{Rk,s}$	[Nm]	8	19	37	66	167	325
	Characteristic bending resistance	property class 8.8	$M^0_{Rk,s}$	[Nm]	12	30	60	105	267	519
	Partial factor		$\gamma_{Ms,V}$	[-]	1,25					
Stainless steel	Characteristic bending resistance A4 / HCR	property class 70	$M^0_{Rk,s}$	[Nm]	11	26	53	92	234	643 [2)]
	Partial factor		$\gamma_{Ms,V}$	[-]	1,56					2,38
Concrete pry-out failure										
Pry-out factor			k_8	[-]	2,0					
Concrete edge failure										
Effective length of anchor			l_f	[mm]	min (h_{ef};12 d_{nom})					min (h_{ef}; 300mm)
Outside diameter of anchor			d_{nom}	[mm]	10	12	16	20	24	30
Installation factor			γ_{inst}	[-]	1,0					

1) Fastening screws or threaded rods (incl. nut and washer) must comply with the appropriate material and property class of the internally threaded anchor rod (exception: VMU-IG M20). The characteristic shear resistance for steel failure of the given strength class are valid for the internally threaded anchor rod and the fastening element

2) For VMU-IG M20: Internally threaded rod: property class 50;
Fastening screws or threaded rods (incl. nut and washer): property class 70

Table C9: Characteristic values of **tension loads** for **rebar** under static or quasi-static action

Reinforcing bar				**Ø 8**	**Ø 10**	**Ø 12**	**Ø 14**	**Ø 16**	**Ø 20**	**Ø 24**	**Ø 25**	**Ø 28**	**Ø 32**
Steel failure													
Characteristic resistance		$N_{Rk,s}$	[kN]	$A_s \cdot f_{uk}$ [1)]									
Cross sectional area		A_s	[mm²]	50	79	113	154	201	314	452	491	616	804
Partial factor		$\gamma_{Ms,N}$	[-]	1,4 [2)]									
Combined pull-out and concrete failure													
Characteristic bond resistance in <u>uncracked</u> concrete C20/25													
Temperature range	I: 80°C / 50°C	$\tau_{Rk,ucr}$	[N/mm²]	14	14	14	14	13	13	13	13	13	13
Temperature range	II: 120°C / 72°C	$\tau_{Rk,ucr}$	[N/mm²]	13	12	12	12	12	11	11	11	11	11
Temperature range	III: 160°C / 100°C	$\tau_{Rk,ucr}$	[N/mm²]	9,5	9,5	9,5	9,0	9,0	9,0	9,0	9,0	8,5	8,5
Characteristic bond resistance in <u>cracked</u> concrete C20/25													
Temperature range	I: 80°C / 50°C	$\tau_{Rk,cr}$	[N/mm²]	5,5	5,5	6,0	6,5	6,5	6,5	6,5	7,0	7,0	7,0
Temperature range	II: 120°C / 72°C	$\tau_{Rk,cr}$	[N/mm²]	4,5	5,0	5,0	5,5	5,5	5,5	5,5	6,0	6,0	6,0
Temperature range	III: 160°C / 100°C	$\tau_{Rk,cr}$	[N/mm²]	4,0	4,5	4,5	5,0	5,0	5,0	5,0	5,0	5,0	5,0
Reduction factor ψ^0_{sus} in concrete C20/25													
Temperature range	I: 80°C / 50°C	ψ^0_{sus}	[-]	0,79									
Temperature range	II: 120°C / 72°C	ψ^0_{sus}	[-]	0,75									
Temperature range	III: 160°C / 100°C	ψ^0_{sus}	[-]	0,66									
Increasing factor for concrete		ψ_c	C25/30	1,02									
			C30/37	1,04									
			C35/45	1,07									
			C40/50	1,08									
			C45/55	1,09									
			C50/60	1,10									
Concrete cone failure													
Factor k_1	uncracked concrete	$k_{ucr,N}$	[-]	11,0									
Factor k_1	cracked concrete	$k_{cr,N}$	[-]	7,7									
Splitting failure													
Edge distance	$h/h_{ef} \geq 2,0$	$c_{cr,sp}$	[mm]	$1,0\ h_{ef}$									
Edge distance	$2,0 > h/h_{ef} > 1,3$	$c_{cr,sp}$	[mm]	$2 \cdot h_{ef}\ (2,5 - h / h_{ef})$									
Edge distance	$h/h_{ef} \leq 1,3$	$c_{cr,sp}$	[mm]	$2,4\ h_{ef}$									
Spacing		$s_{cr,sp}$	[mm]	$2\ c_{cr,sp}$									
Installation factor													
dry or wet concrete	vacuum cleaning	γ_{inst}	[-]	1,2									
dry or wet concrete	manual cleaning	γ_{inst}	[-]	1,2					NPA				
dry or wet concrete	compressed air cleaning	γ_{inst}	[-]	1,0									
waterfilled drill hole	compressed air cleaning	γ_{inst}	[-]	1,4									

[1)] f_{uk} shall be taken from the specifications of reinforcing bars
[2)] in absence of national regulation

Injection System VMH for concrete	**Annex C8**
Performance Characteristic values of **tension loads** for **rebar**	

Table C10: Characteristic values of **shear loads** for **rebar** under static or quasi-static action

Reinforcing bar			Ø 8	Ø 10	Ø 12	Ø 14	Ø 16	Ø 20	Ø 24	Ø 25	Ø 28	Ø 32
Steel failure without lever arm												
Characteristic shear resistance	$V^0_{Rk,s}$	[kN]	$0{,}50 \cdot A_s \cdot f_{uk}$ [1)]									
Cross sectional area	A_s	[mm²]	50	79	113	154	201	314	452	491	616	804
Partial factor	$\gamma_{Ms,V}$	[-]	1,5 [2)]									
Ductility factor	k_7	[-]	1,0									
Steel failure with lever arm												
Characteristic bending resistance	$M^0_{Rk,s}$	[Nm]	$1{,}2 \cdot W_{el} \cdot f_{uk}$ [1)]									
Elastic section modulus	W_{el}	[mm³]	50	98	170	269	402	785	1357	1534	2155	3217
Partial factor	$\gamma_{Ms,V}$	[-]	1,5 [2)]									
Concrete pry-out failure												
Pry-out Factor	k_8	[-]	2,0									
Concrete edge failure												
Effective length of rebar	l_f	[mm]	min (h_{ef};12 d_{nom})							min (h_{ef}; 300mm)		
Outside diameter of rebar	d_{nom}	[mm]	8	10	12	14	16	20	24	25	28	32
Installation factor	γ_{inst}	[-]	1,0									

1) f_{uk} shall be taken from the specifications of reinforcing bars
2) in absence of national regulation

Table C11: Characteristic values of **tension loads** for **rebar** under **seismic action**, performance category **C1**

Reinforcing bar				Ø 8	Ø 10	Ø 12	Ø 14	Ø 16	Ø 20	Ø 24	Ø 25	Ø 28	Ø 32
Steel failure													
Characteristic resistance		$N_{Rk,s,eq,C1}$	[kN]	$A_s \cdot f_{uk}$ 1)									
Cross sectional area		A_s	[mm²]	50	79	113	154	201	314	452	491	616	804
Partial factor		$\gamma_{Ms,N}$	[-]	1,4 2)									
Combined pull-out and concrete failure													
Characteristic bond resistance in concrete C20/25													
Temperature range	I: 80°C / 50°C	$\tau_{Rk,eq,C1}$	[N/mm²]	5,5	5,5	6,0	6,5	6,5	6,5	6,5	7,0	7,0	7,0
	II: 120°C / 72°C	$\tau_{Rk,eq,C1}$	[N/mm²]	4,5	5,0	5,0	5,5	5,5	5,5	5,5	6,0	6,0	6,0
	III: 160°C / 100°C	$\tau_{Rk,eq,C1}$	[N/mm²]	4,0	4,5	4,5	5,0	5,0	5,0	5,0	5,0	5,0	5,0
Installation factor													
dry or wet concrete	vacuum cleaning	γ_{inst}	[-]	1,2									
dry or wet concrete	compressed air cleaning	γ_{inst}	[-]	1,0									
waterfilled drill hole	compressed air cleaning	γ_{inst}	[-]	1,4									

1) f_{uk} shall be taken from the specifications of reinforcing bars
2) in absence of national regulation

Table C12: Characteristic values of **shear loads** for **rebar** under **seismic action**, performance category **C1**

Reinforcing bar			Ø 8	Ø 10	Ø 12	Ø 14	Ø 16	Ø 20	Ø 24	Ø 25	Ø 28	Ø 32
Steel failure without lever arm												
Characteristic resistance	$V^0_{Rk,s,eq,C1}$	[kN]	$0{,}35 \cdot A_s \cdot f_{uk}$ 1)									
Cross sectional area	A_s	[mm²]	50	79	113	154	201	314	452	491	616	804
Partial factor	$\gamma_{Ms,V}$	[-]	1,5 2)									
Ductility factor	k_7	[-]	1,0									
Steel failure with lever arm												
Characteristic bending resistance	$M^0_{Rk,s,eq,C1}$	[Nm]	No Performance Assessd (NPA)									
Installation factor	γ_{inst}	[-]	1,0									

1) f_{uk} shall be taken from the specifications of reinforcing bars
2) in absence of national regulation

Table C13: **Displacements** under **tension load** (threaded rod)

Threaded rod			**M8**	**M10**	**M12**	**M16**	**M20**	**M24**	**M27**	**M30**
Displacement factor[1)] for uncracked concrete under static and quasi-static action										
Temperature range I: 80°C / 50°C	δ_{N0}-factor	[mm/(N/mm²)]	0,031	0,032	0,034	0,037	0,039	0,042	0,044	0,046
	$\delta_{N\infty}$-factor	[mm/(N/mm²)]	0,040	0,042	0,044	0,047	0,051	0,054	0,057	0,060
Temperature range II: 120°C / 72°C	δ_{N0}-factor	[mm/(N/mm²)]	0,032	0,034	0,035	0,038	0,041	0,044	0,046	0,048
	$\delta_{N\infty}$-factor	[mm/(N/mm²)]	0,042	0,044	0,045	0,049	0,053	0,056	0,059	0,062
Temperature range III: 160°C / 100°C	δ_{N0}-factor	[mm/(N/mm²)]	0,121	0,126	0,131	0,142	0,153	0,163	0,171	0,179
	$\delta_{N\infty}$-factor	[mm/(N/mm²)]	0,124	0,129	0,135	0,146	0,157	0,168	0,176	0,184
Displacement factor[1)] for cracked concrete under static and quasi-static action										
Temperature range I: 80°C / 50°C	δ_{N0}-factor	[mm/(N/mm²)]	0,081	0,083	0,085	0,090	0,095	0,099	0,103	0,106
	$\delta_{N\infty}$-factor	[mm/(N/mm²)]	0,104	0,107	0,110	0,116	0,122	0,128	0,133	0,137
Temperature range II: 120°C / 72°C	δ_{N0}-factor	[mm/(N/mm²)]	0,084	0,086	0,088	0,093	0,098	0,103	0,107	0,110
	$\delta_{N\infty}$-factor	[mm/(N/mm²)]	0,108	0,111	0,114	0,121	0,127	0,133	0,138	0,143
Temperature range III: 160°C / 100°C	δ_{N0}-factor	[mm/(N/mm²)]	0,312	0,321	0,330	0,349	0,367	0,385	0,399	0,412
	$\delta_{N\infty}$-factor	[mm/(N/mm²)]	0,321	0,330	0,340	0,358	0,377	0,396	0,410	0,424
Cracked concrete under seismic action (C2)										
All temperature ranges	$\delta_{N,eq\ (DLS)}$	[mm]	NPA		0,24	0,27	0,29	0,27	NPA	
	$\delta_{N,eq\ (ULS)}$	[mm]			0,55	0,51	0,50	0,58		

[1)] Calculation of the displacement
$\delta_{N0} = \delta_{N0}$- factor $\cdot\ \tau$; τ: acting bond stress for tension
$\delta_{N\infty} = \delta_{N\infty}$- factor $\cdot\ \tau$;

Table C14: **Displacements** under **shear load** (threaded rod)

Threaded rod			**M 8**	**M 10**	**M 12**	**M 16**	**M 20**	**M24**	**M 27**	**M 30**
Displacement factor[1)] for concrete under static and quasi-static action										
All temperature ranges	δ_{V0}-Faktor	[mm/(kN)]	0,06	0,06	0,05	0,04	0,04	0,03	0,03	0,03
	$\delta_{V\infty}$-Faktor	[mm/(kN)]	0,09	0,08	0,08	0,06	0,06	0,05	0,05	0,05
Cracked concrete under seismic action (C2)										
All temperature ranges	$\delta_{V,eq(DLS)}$	[mm]	NPA		3,6	3,0	3,1	3,5	NPA	
	$\delta_{V,eq(ULS)}$	[mm]			7,0	6,6	7,0	9,3		

[1)]Calculation of the displacement
$\delta_{V0} = \delta_{V0}$-factor $\cdot$ V; V: acting shear load
$\delta_{V\infty} = \delta_{V\infty}$-factor $\cdot$ V;

Table C15: **Displacements** under **tension load** (internally threaded anchor rod)

Internally threaded anchor rod			**VMU-IG M 6**	**VMU-IG M 8**	**VMU-IG M 10**	**VMU-IG M 12**	**VMU-IG M 16**	**VMU-IG M 20**
Displacement factor[1] for uncracked concrete under static and quasi-static action								
Temperature range I: 80°C / 50°C	δ_{N0}-factor	[mm/(N/mm²)]	0,032	0,034	0,037	0,039	0,042	0,046
	$\delta_{N\infty}$-factor	[mm/(N/mm²)]	0,042	0,044	0,047	0,051	0,054	0,060
Temperature range II: 120°C / 72°C	δ_{N0}-factor	[mm/(N/mm²)]	0,034	0,035	0,038	0,041	0,044	0,048
	$\delta_{N\infty}$-factor	[mm/(N/mm²)]	0,044	0,045	0,049	0,053	0,056	0,062
Temperature range III: 160°C / 100°C	δ_{N0}-factor	[mm/(N/mm²)]	0,126	0,131	0,142	0,153	0,163	0,179
	$\delta_{N\infty}$-factor	[mm/(N/mm²)]	0,129	0,135	0,146	0,157	0,168	0,184
Displacement factor[1] for cracked concrete under static and quasi-static action								
Temperature range I: 80°C / 50°C	δ_{N0}-factor	[mm/(N/mm²)]	0,083	0,085	0,090	0,095	0,099	0,106
	$\delta_{N\infty}$-factor	[mm/(N/mm²)]	0,107	0,110	0,116	0,122	0,128	0,137
Temperature range II: 120°C / 72°C	δ_{N0}-factor	[mm/(N/mm²)]	0,086	0,088	0,093	0,098	0,103	0,110
	$\delta_{N\infty}$-factor	[mm/(N/mm²)]	0,111	0,114	0,121	0,127	0,133	0,143
Temperature range III: 160°C / 100°C	δ_{N0}-factor	[mm/(N/mm²)]	0,321	0,330	0,349	0,367	0,385	0,412
	$\delta_{N\infty}$-factor	[mm/(N/mm²)]	0,330	0,340	0,358	0,377	0,396	0,424

[1] Calculation of the displacement
$\delta_{N0} = \delta_{N0}$-factor $\cdot \tau$; τ: acting bond stress for tension
$\delta_{N\infty} = \delta_{N\infty}$-factor $\cdot \tau$;

Table C16: **Displacements** under **shear load** (internally threaded anchor rod)

Internally threaded anchor rod			**VMU-IG M 6**	**VMU-IG M 8**	**VMU-IG M 10**	**VMU-IG M 12**	**VMU-IG M 16**	**VMU-IG M 20**
Displacement factor[1] under static and quasi-static action								
All temperature ranges	δ_{V0}-factor	[mm/(kN)]	0,07	0,06	0,06	0,05	0,04	0,04
	$\delta_{V\infty}$-factor	[mm/(kN)]	0,10	0,09	0,08	0,08	0,06	0,06

[1] Calculation of the displacement
$\delta_{V0} = \delta_{V0}$-factor $\cdot V$;   V: acting shear load
$\delta_{V\infty} = \delta_{V\infty}$-factor $\cdot V$;

Table C17: **Displacements** under **tension load** (rebar)

Rebar			**Ø 8**	**Ø 10**	**Ø 12**	**Ø 14**	**Ø 16**	**Ø 20**	**Ø 24**	**Ø 25**	**Ø 28**	**Ø 32**
Displacement factor[1] for uncracked concrete under static and quasi-static action												
Temperature range I: 80°C / 50°C	δ_{N0}-factor	[mm/(N/mm²)]	0,031	0,032	0,034	0,035	0,037	0,039	0,042	0,043	0,045	0,048
	$\delta_{N\infty}$-factor	[mm/(N/mm²)]	0,040	0,042	0,044	0,045	0,047	0,051	0,054	0,055	0,058	0,063
Temperature range II: 120°C / 72°C	δ_{N0}-factor	[mm/(N/mm²)]	0,032	0,034	0,035	0,036	0,038	0,041	0,044	0,045	0,047	0,050
	$\delta_{N\infty}$-factor	[mm/(N/mm²)]	0,042	0,044	0,045	0,047	0,049	0,053	0,056	0,057	0,060	0,065
Temperature range III: 160°C / 100°C	δ_{N0}-factor	[mm/(N/mm²)]	0,121	0,126	0,131	0,137	0,142	0,153	0,163	0,164	0,172	0,186
	$\delta_{N\infty}$-factor	[mm/(N/mm²)]	0,124	0,129	0,135	0,141	0,146	0,157	0,168	0,169	0,177	0,192
Displacement factor[1] for cracked concrete under static and quasi-static action												
Temperature range I: 80°C / 50°C	δ_{N0}-factor	[mm/(N/mm²)]	0,081	0,083	0,085	0,087	0,090	0,095	0,099	0,099	0,103	0,108
	$\delta_{N\infty}$-factor	[mm/(N/mm²)]	0,104	0,107	0,110	0,113	0,116	0,122	0,128	0,128	0,133	0,141
Temperature range II: 120°C / 72°C	δ_{N0}-factor	[mm/(N/mm²)]	0,084	0,086	0,088	0,090	0,093	0,098	0,103	0,103	0,107	0,113
	$\delta_{N\infty}$-factor	[mm/(N/mm²)]	0,108	0,111	0,114	0,118	0,121	0,127	0,133	0,133	0,138	0,148
Temperature range III: 160°C / 100°C	δ_{N0}-factor	[mm/(N/mm²)]	0,312	0,321	0,330	0,340	0,349	0,367	0,385	0,385	0,399	0,425
	$\delta_{N\infty}$-factor	[mm/(N/mm²)]	0,321	0,330	0,340	0,349	0,358	0,377	0,396	0,396	0,410	0,449

[1] Calculation of the displacement
$\delta_{N0} = \delta_{N0}\text{-factor} \cdot \tau$; τ: acting bond stress for tension
$\delta_{N\infty} = \delta_{N\infty}\text{- factor} \cdot \tau$;

Table C18: **Displacements** under **shear load** (rebar)

Rebar			**Ø 8**	**Ø 10**	**Ø 12**	**Ø 14**	**Ø 16**	**Ø 20**	**Ø 24**	**Ø 25**	**Ø 28**	**Ø 32**
Displacement factor[1] under static and quasi-static action												
All temperature ranges	δ_{V0}-factor	[mm/(kN)]	0,06	0,05	0,05	0,04	0,04	0,04	0,03	0,03	0,03	0,03
	$\delta_{V\infty}$-factor	[mm/(kN)]	0,09	0,08	0,08	0,06	0,06	0,05	0,05	0,05	0,04	0,04

[1] Calculation of the displacement
$\delta_{V0} = \delta_{V0}\text{-factor} \cdot V$; V: acting shear load
$\delta_{V\infty} = \delta_{V\infty}\text{-factor} \cdot V$;