...eine starke Verbindung

YDEEVNEDEKLARATION

DoP Nr.: **MKT-2.1-301**_da

✧ **Varetypens unikke identifikationskode:** **Injektionssystem VMU plus til beton**

✧ **Tilsigtet anvendelse:** Bundet anker til forankring i beton, se bilag / Annex B

✧ **Fabrikant:** MKT Metall-Kunststoff-Technik GmbH & Co.KG
Auf dem Immel 2
67685 Weilerbach

✧ **System eller systemer til vurdering og kontrol af konstansen af ydeevnen:** 1

✧ **Europæisk vurderingsdokument:** **EAD 330499-01-0601**
Europæisk teknisk vurdering: **ETA-11/0415, 01.06.2021**
Teknisk vurderingsorgan: DIBt, Berlin
Notificeret organ/notificerede organer: NB 2873 – Technische Universität Darmstadt

✧ **Deklareret ydeevne/deklarerede ydeevner:**

Væsentlige funktioner	Ydeevne
Mekanisk modstandsdygtighed og stabilitet (BWR 1)	
Karakteristiske modstande under trækbelastning (statiske og kvasistatiske effekter)	Bilag / Annex B2, C1, C3, C4, C7, C9
Karakteristiske modstande under tværgående stress (statiske og kvasistatiske effekter)	Bilag / Annex C2, C5, C8, C10
Forskydninger (statiske og kvasistatiske effekter)	Bilag / Annex C12, C13
Karakteristisk modstand og forskydninger for seismisk ydeevne kategori C1	Bilag / Annex C6, C11
Karakteristisk modstand og forskydninger for seismisk ydeevne kategori C2	Ydeevne ikke bedømt
Hygiejne, sundhed og miljø (BWR 3)	
Indhold, emission og / eller frigivelse af farlige stoffer	Ydeevne ikke bedømt

Ydeevnen for den vare, der er anført ovenfor, er i overensstemmelse med den deklarerede ydeevne. Denne ydeevnedeklaration er udarbejdet i overensstemmelse med forordning (EU) nr. 305/2011 på eneansvar af den fabrikant, der er anført ovenfor.

Underskrevet for fabrikanten og på dennes vegne af:

Stefan Weustenhagen
(CEO)
Weilerbach, 01.06.2021

p.p.
Dipl.-Ing. Detlef Bigalke
(Leder af produktudvikling)

ETA

Originalen af denne erklæringserklæring blev skrevet på tysk. I tilfælde af afvigelser i oversættelsen er den tyske udgave gyldig.

Specification of intended use

Injection System VMU plus	Threaded rod	Internally threaded anchor rod	Rebar
Static and quasi-static action	M8 - M30	IG-M6 - IG-M20 (zinc plated, A4, HCR)	Ø8 - Ø32
Seismic action, performance category C1	M8 - M30	-	Ø8 - Ø32
Base materials	compacted, reinforced or unreinforced normal weight concrete (without fibers), acc. to EN 206:2013 + A1:2016 strength classes C20/25 to C50/60 acc. to EN 206-1:2013+A1:2016 cracked and uncracked concrete		
Temperature Range I -40°C to +40°C	max long term temperature +24 °C and max short term temperature +40°C		
Temperature Range II -40°C to +80°C	max long term temperature +50 °C and max short term temperature +80°C		
Temperature Range III -40°C to 120°C	max long term temperature +72 °C and max short term temperature +120°C		

Use conditions (Environmental conditions):

- Structures subject to dry internal conditions (all materials).
- For all other conditions:
  Intended use of Material according to Annx A4, Table A1 corresponding corrosion resistance classes CRC according to EN 1993-1-4:2006 +A1:2015

Design:

- Verifiable calculation notes and drawings are prepared taking account of the loads to be anchored. The position of the anchor is indicated on the design drawings (e. g. position of the anchor relative to reinforcement or to supports, etc.)
- Anchorages are designed under the responsibility of an engineer experienced in anchorages and concrete work
- Anchorages are designed in accordance with EN 1992-4:2018 and Technical Report TR 055, Edition February 2018

Installation:

- Dry or wet concrete: M8 to M30, IG-M6 to IG-M20, Rebar Ø8 to Ø32
- Waterfilled holes (not sea water): M8 to M16, IG-M6 to IG-M10, Rebar Ø8 to Ø16
- Hole drilling by hammer or compressed air drill mode or vacuum drill mode
- Installation direction D3: downwards, horizontally and upwards (overhead) installation
- Anchor installation carried out by appropriately qualified personnel and under the supervision of the person responsible for technical matters of the site
- The injection mortar is assessed for installation at minimum concrete temperature of -10°C resp. -20°C, where subsequently the temperature in the concrete does not rise at a rapid rate, i.e. from the minimum installation temperature to 24°C within a 12-hour period.
- Internally threaded anchor rod: screws and threaded rods (incl. nut and washer) must at least correspond to the material and strength class of the internally threaded anchor rod used

Table B1: Installation parameters for threaded rod

Threaded rod				M8	M10	M12	M16	M20	M24	M27	M30
Diameter threaded rod		$d=d_{nom}$	[mm]	8	10	12	16	20	24	27	30
Nominal drill hole diameter		d_0	[mm]	10	12	14	18	24	28	32	35
Effective anchorage depth		$h_{ef,min}$	[mm]	60	60	70	80	90	96	108	120
		$h_{ef,max}$	[mm]	160	200	240	320	400	480	540	600
Diameter of clearance hole in the fixture	Pre-setting installation	$d_f \leq$	[mm]	9	12	14	18	22	26	30	33
	Through setting installation	$d_f \leq$	[mm]	12	14	16	20	25	30	33	38
Installation torque		max $T_{inst} \leq$	[Nm]	10	20	40 (35)[1]	80	120	160	180	200
Minimum thickness of member		h_{min}	[mm]	h_{ef} + 30mm ≥ 100mm			$h_{ef} + 2d_0$				
Minimum spacing		s_{min}	[mm]	40	50	60	80	100	120	135	150
Minimum edge distance		c_{min}	[mm]	40	50	60	80	100	120	135	150

[1] max. installation torque for property class 4.6

Table B2: Installation parameters for internally threaded anchor rod

Internally threaded anchor rod			IG-M 6	IG-M 8	IG-M 10	IG-M 12	IG-M 16	IG-M 20
Inner diameter of threaded rod	d_2	[mm]	6	8	10	12	16	20
Outer diameter of threaded rod[1]	$d=d_{nom}$	[mm]	10	12	16	20	24	30
Nominal drill hole diameter	d_0	[mm]	12	14	18	24	28	35
Effective anchorage depth	$h_{ef,min}$	[mm]	60	70	80	90	96	120
	$h_{ef,max}$	[mm]	200	240	320	400	480	600
Diameter of clearance hole in the fixture	$d_f \leq$	[mm]	7	9	12	14	18	22
Installation torque	max $T_{inst} \leq$	[Nm]	10	10	20	40	60	100
Minimum screw-in depth	l_{IG}	[mm]	8	8	10	12	16	20
Minimum thickness of member	h_{min}	[mm]	h_{ef} + 30 mm ≥ 100 mm		$h_{ef} + 2d_0$			
Minimum spacing	s_{min}	[mm]	50	60	80	100	120	150
Minimum edge distance	c_{min}	[mm]	50	60	80	100	120	150

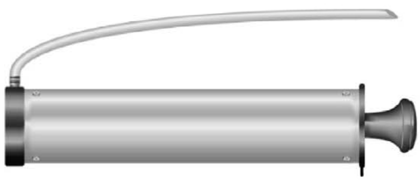
[1] with metric thread acc. to EN 1993-1-8:2005+AC:2009

Table B3: Installation parameters for rebar

Rebar			Ø 8	Ø 10	Ø 12	Ø 14	Ø 16	Ø 20	Ø 25	Ø 28	Ø 32
Diameter threaded rod	$d=d_{nom}$	[mm]	8	10	12	14	16	20	24	28	32
Nominal drill hole diameter	d_0	[mm]	12	14	16	18	20	24	32	35	40
Effective anchorage depth	$h_{ef,min}$	[mm]	60	60	70	75	80	90	100	112	128
	$h_{ef,max}$	[mm]	160	200	240	280	320	400	500	560	640
Minimum thickness of member	h_{min}	[mm]	h_{ef} + 30 mm ≥ 100 mm		$h_{ef} + 2d_0$						
Minimum spacing	s_{min}	[mm]	40	50	60	70	80	100	125	140	160
Minimum edge distance	c_{min}	[mm]	40	50	60	70	80	100	125	140	160

Table B4: Parameter cleaning and setting tools

Threaded rod	Internally threaded anchor rod	Rebar	Drill bit Ø	Brush Ø	min. Brush Ø	Retaining washer			
							Installation direction and use of retaining washer		
[-]	[-]	Ø [mm]	d_0 [mm]	d_b [mm]	$d_{b,min}$ [mm]	[-]	⬇	➡	⬆
M8			10	12	10,5	No **retaining washer** required			
M10	VMU-IG M 6	8	12	14	12,5				
M12	VMU-IG M 8	10	14	16	14,5				
		12	16	18	16,5				
M16	VMU-IG M10	14	18	20	18,5	VM-IA 18			
		16	20	22	20,5	VM-IA 20			
M20	VMU-IG M12	20	24	26	24,5	VM-IA 24			
M24	VMU-IG M16		28	30	28,5	VM-IA 28	h_{ef} > 250mm	h_{ef} > 250mm	all
M27		25	32	34	32,5	VM-IA 32			
M30	VMU-IG M20	28	35	37	35,5	VM-IA 35			
		32	40	41,5	40,5	VM-IA 40			



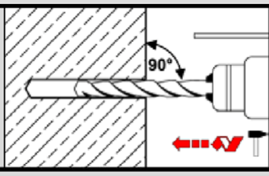
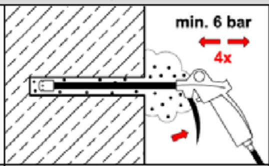
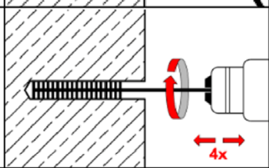
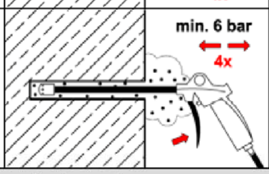
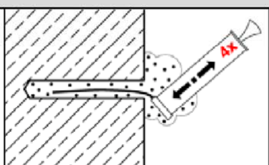
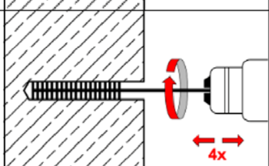
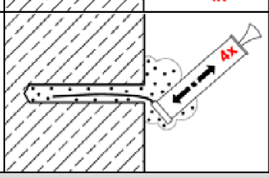
Blow-out pump (volume 750ml)
Drill bit diameter (d_0): 10 mm to 20 mm
Anchorage depth (h_{ef}): ≤ 10 d_{nom}
for uncracked concrete

Recommended compressed air tool (min 6 bar)
All applications

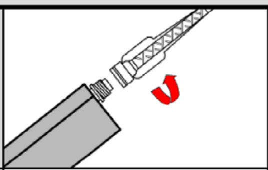
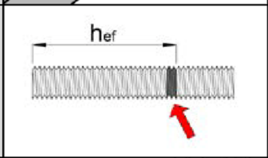
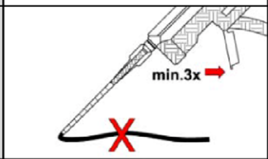
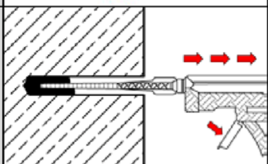
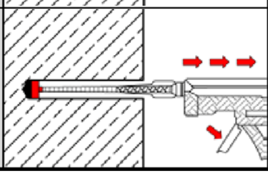
Retaining washer for overhead or horizontal installation
Drill bit diameter (d_0):
18 mm to 40 mm

Steel brush
Drill bit diameter (d_0): all diameters

Installation instructions

Drilling of the hole		
1		Drill the hole by applying the drilling method acc. to Annex B1, the drill bit diameter (Table B4) and the selected drill hole depth. In case of aborted hole, the drill hole shall be filled with mortar
Cleaning, all drilling methods		
2	**Attention! Standing water in the drill hole must be removed before cleaning!**	
	<u>**Cleaning with compressed air**</u> (all diameters, cracked and uncracked concrete)	
2a		Starting from the bottom or back of the drill hole, blow out the hole with compressed air (min. 6 bar) **four** times, until return air stream is free of noticeable dust. If the drill hole ground is not reached, an extension must be used.
2b		Brush the hole with an appropriate sized wire brush > $d_{b,min}$ (Table B4) **four** times. If the drill hole ground is not reached, a brush extension shall be used.
2c		Finally blow the hole clean again with compressed air (min. 6 bar) **four** times, until the outgoing airstream is free of dust. If the drill hole ground is not reached an extension shall be used.
	<u>**Manual cleaning**</u> <u>uncracked concrete:</u> Drill hole diameter $d_0 \leq$ 20mm and effective anchorage depth $h_{ef} \leq 10\ d_{nom}$ <u>cracked concrete:</u> Drill hole diameter: 14mm $\leq d_0 \leq$ 20mm and effective anchorage depth $h_{ef} \leq 10\ d_{nom}$	
2a		Starting from the bottom or back of the drill hole, blow the hole clean with the blow-out pump **four** times until retur air stream is free of noticeable dust.
2b		Brush the hole **four** times with an appropriate sized wire brush > $d_{b,min}$ (Table B4). If the drill hole ground is not reached, a brush extension shall be used.
2c		Finally blow the hole clean again with the blow-out pump **four** times until retur air stream is free of noticeable dust.
After cleaning, the drill hole has to be protected against re-contamination in an appropriate way, until dispensing the mortar in the drill hole. If necessary, the cleaning has to be repeated directly before dispensing the mortar. In-flowing water must not contaminate the drill hole again.		

Installation instructions (continuation)

Injection		
3		Attach a supplied static-mixing nozzle to the cartridge and load the cartridge into the correct dispensing tool. For every working interruption longer than the recommended working time (Table B5 or Table B6) as well as for new cartridges, a new static-mixer shall be used.
4		Before injecting the mortar, mark the required anchorage depth on the fastening element.
5		Prior to dispensing into the drill hole, squeeze out separately a minimum of three full strokes and discard non-uniformly mixed adhesive components until the mortar shows a consistent grey colour. For tubular film cartridges dismiss a minimum of six full strokes.
6a		Starting from the bottom or back of the cleaned drill hole fill the hole up to approximately two-thirds with adhesive. Slowly withdraw the static mixing nozzle as the hole fills to avoid air pockets. For embedment larger than 190mm an extension nozzle shall be used. Observe the gel-/ working times given in Table B5 or Table B6.
6b		Retaining washer and mixer nozzle extensions shall be used according to Annex B3 for the following applications: • Horizontal installation (horizontal direction) and ground installation (vertical downwards direction): Drill bit-Ø $d_0 \geq 18$ mm and embedment depth $h_{ef} > 250$mm • Overhead installation: Drill bit-Ø $d_0 \geq 18$ mm

Installation instructions (continuation)

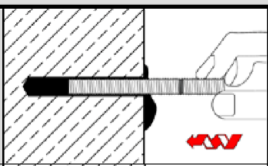
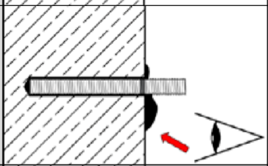
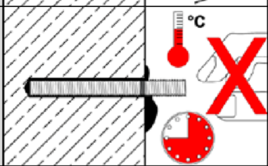
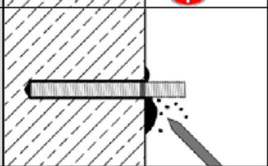
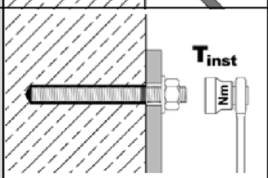
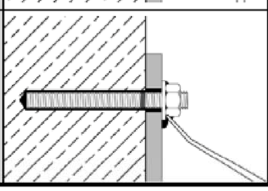
Setting the fastening element		
7		Push fastening element into the hole while turning slightly to ensure proper distribution of the adhesive until the embedment depth is reached. The anchor shall be free of dirt, grease, oil or other foreign material.
8		Make sure that the fastening element is fully seated up to the full embedment depth and that excess mortar is visible at the top of the hole. If these requirements are not maintained, the application has to be renewed before the end of the working time. For overhead installation, the anchor should be fixed (e.g. by wedges).
9		Allow the adhesive to cure to the specified time prior to applying any load or torque. Do not move or load the anchor until it is fully cured (Table B5 or Table B6).
10		Remove excess mortar.
11		The fixture can be mounted after curing time. Apply installation torque ≤ T_{inst} according to Table B1or B2.
12		Optionally, for pre-setting installation, the annular gap between anchor rod and attachment can be filled with mortar. Therefor replace the regular washer by washer with drill and plug on reducing adapter on static mixer. Annular gap is completely filled, when excess mortar seeps out.

Table B5: Maximum processing time and minimum curing time, VMU plus

Concrete temperature	Maximum processing time	Minimum curing time in dry concrete[1)]
- 10°C to - 6°C	90 min[2)]	24 h[2)]
- 5°C to - 1°C	90 min	14 h
0°C to + 4°C	45 min	7 h
+ 5°C to + 9°C	25 min	2 h
+ 10°C to + 19°C	15 min	80 min
+ 20°C to + 29°C	6 min	45 min
+ 30°C to + 34°C	4 min	25 min
+ 35°C to + 39°C	2 min	20 min
+ 40°C	1,5 min	15 min
Cartridge temperature	**+ 5°C to + 40°C**	

1) in wet concrete the curing time must be doubled
2) cartridge temperature must be at min. +15°C

Table B6: Maximum processing time and minimum curing time, VMU plus Polar

Concrete temperature	Maximum processing time	Minimum curing time in dry concrete[1)]
- 20°C to - 16°C	75 min	24 h
- 15°C to - 11°C	55 min	16 h
- 10°C to - 6°C	35 min	10 h
- 5°C to - 1°C	20 min	5 h
0°C to + 4°C	10 min	2,5 h
+ 5°C to + 9°C	6 min	80 min
+10°C	6 min	60 min
Cartridge temperature	**- 20°C to + 10°C**	

1) in wet concrete the curing time must be doubled

Table C1: Characteristic **steel resistances** for **threaded rods** under **tension loads**

Threaded rod				M8	M10	M12	M16	M20	M24	M27	M30
Steel failure											
Cross sectional area		A_s	[mm²]	36,6	58,0	84,3	157	245	353	459	561
Characteristic resistance under tension load [1]											
Steel, zinc plated	Property class 4.6 and 4.8	$N_{Rk,s}$	[kN]	15 (13)	23 (21)	34	63	98	141	184	224
Steel, zinc plated	Property class 5.6 and 5.8	$N_{Rk,s}$	[kN]	18 (17)	29 (27)	42	78	122	176	230	280
Steel, zinc plated	Property class 8.8	$N_{Rk,s}$	[kN]	29 (27)	46 (43)	67	125	196	282	368	449
Stainless steel	A2, A4 and HCR Property class 50	$N_{Rk,s}$	[kN]	18	29	42	79	123	177	230	281
Stainless steel	A2, A4 and HCR Property class 70	$N_{Rk,s}$	[kN]	26	41	59	110	171	247	-[3]	-[3]
Stainless steel	A4 and HCR Property class 80	$N_{Rk,s}$	[kN]	29	46	67	126	196	282	-[3]	-[3]
Partial factors [2]											
Steel, zinc plated	Property class 4.6	$\gamma_{Ms,N}$	[-]	2,0							
Steel, zinc plated	Property class 4.8	$\gamma_{Ms,N}$	[-]	1,5							
Steel, zinc plated	Property class 5.6	$\gamma_{Ms,N}$	[-]	2,0							
Steel, zinc plated	Property class 5.8	$\gamma_{Ms,N}$	[-]	1,5							
Steel, zinc plated	Property class 8.8	$\gamma_{Ms,N}$	[-]	1,5							
Stainless steel	A2, A4 and HCR Property class 50	$\gamma_{Ms,N}$	[-]	2,86							
Stainless steel	A2, A4 and HCR Property class 70	$\gamma_{Ms,N}$	[-]	1,87						-[3]	-[3]
Stainless steel	A4 and HCR Property class 80	$\gamma_{Ms,N}$	[-]	1,6						-[3]	-[3]

[1] the characteristic resistances apply for all anchor rods with the cross sectional area A_s specified here: VMU-A, V-A, VM-A
For commercial standard threaded rods with a smaller cross sectional area (e.g. hot-dip galvanized threaded rods M8, M10 according to EN ISO 10684:2004 + AC:2009), the values in brackets are valid.

[2] in absence of national regulation

[3] Anchor type not part of the ETA

Table C2: Characteristic **steel resistances** for **threaded rods** under **shear loads**

Threaded rod				**M8**	**M10**	**M12**	**M16**	**M20**	**M24**	**M27**	**M30**
Steel failure											
Cross sectional area		A_s	[mm²]	36,6	58,0	84,3	157	245	353	459	561
Characteristic resistance under shear load [1)]											
Steel failure <u>without</u> lever arm											
Steel, zinc plated	Property class 4.6 and 4.8	$V^0_{Rk,s}$	[kN]	9 (8)	14 (13)	20	38	59	85	110	135
	Property class 5.6 and 5.8	$V^0_{Rk,s}$	[kN]	11 (10)	17 (16)	25	47	74	106	138	168
	Property class 8.8	$V^0_{Rk,s}$	[kN]	15 (13)	23 (21)	34	63	98	141	184	224
Stainless steel	A2, A4 and HCR, property class 50	$V^0_{Rk,s}$	[kN]	9	15	21	39	61	88	115	140
	A2, A4 and HCR, property class 70	$V^0_{Rk,s}$	[kN]	13	20	30	55	86	124	-[3)]	-[3)]
	A4 and HCR, property class 80	$V^0_{Rk,s}$	[kN]	15	23	34	63	98	141	-[3)]	-[3)]
Steel failure <u>with</u> lever arm											
Steel, zinc plated	Property class 4.6 and 4.8	$M^0_{Rk,s}$	[Nm]	15 (13)	30 (27)	52	133	260	449	666	900
	Property class 5.6 and 5.8	$M^0_{Rk,s}$	[Nm]	19 (16)	37 (33)	65	166	324	560	833	1123
	Property class 8.8	$M^0_{Rk,s}$	[Nm]	30 (26)	60 (53)	105	266	519	896	1333	1797
Stainless steel	A2, A4 and HCR, property class 50	$M^0_{Rk,s}$	[Nm]	19	37	66	167	325	561	832	1125
	A2, A4 and HCR, property class 70	$M^0_{Rk,s}$	[Nm]	26	52	92	232	454	784	-[3)]	-[3)]
	A4 and HCR, property class 80	$M^0_{Rk,s}$	[Nm]	30	59	105	266	519	896	-[3)]	-[3)]
Partial factor [2)]											
Steel, zinc plated	Property class 4.6	$\gamma_{Ms,V}$	[-]	1,67							
	Property class 4.8	$\gamma_{Ms,V}$	[-]	1,25							
	Property class 5.6	$\gamma_{Ms,V}$	[-]	1,67							
	Property class 5.8	$\gamma_{Ms,V}$	[-]	1,25							
	Property class 8.8	$\gamma_{Ms,V}$	[-]	1,25							
Stainless steel	A2, A4 and HCR, property class 50	$\gamma_{Ms,V}$	[-]	2,38							
	A2, A4 and HCR, property class 70	$\gamma_{Ms,V}$	[-]	1,56						-[3)]	-[3)]
	A4 and HCR, property class 80	$\gamma_{Ms,V}$	[-]	1,33						-[3)]	-[3)]

[1)] the characteristic resistances apply for all anchor rods with the cross sectional area A_s specified here: VMU-A, V-A, VM-A
For commercial standard threaded rods with a smaller cross sectional area (e.g. hot-dip galvanized threaded rods M8, M10 according to EN ISO 10684:2004 + AC:2009), the values in brackets are valid.

[2)] in absence of national regulation

[3)] Anchor type not part of the ETA

Table C3: Characteristic values for **concrete cone** and **splitting failure**

Threaded rods / Internally threaded anchor rods / Rebars				**all sizes**
Concrete cone failure				
Factor k_1	uncracked concrete	$k_{ucr,N}$	[-]	11,0
	cracked concrete	$k_{cr,N}$	[-]	7,7
Edge distance		$c_{cr,N}$	[mm]	$1,5 \cdot h_{ef}$
Spacing		$s_{cr,N}$	[mm]	$2 \cdot c_{cr,N}$
Splitting failure				
Characteristic resistance		$N^0_{Rk,sp}$	[kN]	min ($N_{Rk,p}$; $N^0_{Rk,c}$)
Edge distance	$h/h_{ef} \geq 2,0$	$c_{cr,sp}$	[mm]	$1,0 \cdot h_{ef}$
	$2,0 > h/h_{ef} > 1,3$			$2 \cdot h_{ef} (2,5 - h / h_{ef})$
	$h/h_{ef} \leq 1,3$			$2,4 \cdot h_{ef}$
Spacing		$s_{cr,sp}$	[mm]	$2 \cdot c_{cr,sp}$

Table C4: Characteristic values for **threaded rods** under **tension loads**

Threaded rod					M8	M10	M12	M16	M20	M24	M27	M30
Steel failure												
Characteristic resistance			$N_{Rk,s}$	[kN]	$A_s \cdot f_{uk}$ (or see Table C1)							
Partial factor			$\gamma_{Ms,N}$	[-]	see Table C1							
Combined pull-out and concrete failure												
Characteristic bond resistance in uncracked concrete C20/25												
Temperature range	I: 40°C/24°C	dry or wet concrete	$\tau_{Rk,ucr}$	[N/mm²]	10	12	12	12	12	11	10	9
	II: 80°C/50°C				7,5	9	9	9	9	8,5	7,5	6,5
	III: 120°C/72°C				5,5	6,5	6,5	6,5	6,5	6,5	5,5	5,0
	I: 40°C/24°C	waterfilled drill hole	$\tau_{Rk,ucr}$	[N/mm²]	7,5	8,5	8,5	8,5	no performance assessed			
	II: 80°C/50°C				5,5	6,5	6,5	6,5				
	III: 120°C/72°C				4,0	5,0	5,0	5,0				
Characteristic bond resistance in cracked concrete C20/25												
Temperature range	I: 40°C/24°C	dry or wet concrete	$\tau_{Rk,cr}$	[N/mm²]	4,0	5,0	5,5	5,5	5,5	5,5	6,5	6,5
	II: 80°C/50°C				2,5	3,5	4,0	4,0	4,0	4,0	4,5	4,5
	III: 120°C/72°C				2,0	2,5	3,0	3,0	3,0	3,0	3,5	3,5
	I: 40°C/24°C	waterfilled drill hole	$\tau_{Rk,cr}$	[N/mm²]	4,0	4,0	5,5	5,5	no performance assessed			
	II: 80°C/50°C				2,5	3,0	4,0	4,0				
	III: 120°C/72°C				2,0	2,5	3,0	3,0				
Reductionfactor ψ^0_{sus} in concrete C20/25												
Temperature range	I: 40°C/24°C	dry or wet concrete; waterfilled drill hole	ψ^0_{sus}	[-]	0,73							
	II: 80°C/50°C				0,65							
	III: 120°C/72°C				0,57							
Increasing factors for τ_{Rk}			ψ_c	C25/30	1,02							
				C30/37	1,04							
				C35/45	1,07							
				C40/50	1,08							
				C45/55	1,09							
				C50/60	1,10							
Concrete cone failure												
Relevant parameter					see Table C3							
Splitting failure												
Relevant parameter					see Table C3							
Installation factor												
dry or wet concrete			γ_{inst}	[-]	1,0	1,2						
waterfilled drill hole			γ_{inst}	[-]	1,4				no performance assessed			

Injection system VMU plus for concrete

Performance
Characteristic values for **threaded rods** under **tension loads**

Annex C4

Table C5: Characteristic values for **threaded rods** under **shear loads**

Threaded rod			**M8**	**M10**	**M12**	**M16**	**M20**	**M24**	**M27**	**M30**
Steel failure without lever arm										
Characteristic resistance, steel zinc plated, property class 4.6, 4.8, 5.6, 5.8	$V^0_{Rk,s}$	[kN]	$0{,}6 \cdot A_s \cdot f_{uk}$ (or see table C2)							
Characteristic resistance, steel zinc plated, property class 8.8, stainless steel A2 / A4 / HCR, all property classes	$V^0_{Rk,s}$	[kN]	$0{,}5 \cdot A_s \cdot f_{uk}$ (or see table C2)							
Ductility factor	k_7	[-]	1,0							
Partial factor	$\gamma_{Ms,V}$	[-]	see Table C2							
Steel failure with lever arm										
Characteristic bending moment	$M^0_{Rk,s}$	[Nm]	$1{,}2 \cdot W_{el} \cdot f_{uk}$ (or see table C2)							
Elastic section modulus	W_{el}	[mm³]	31	62	109	277	541	935	1387	1874
Partial factor	$\gamma_{Ms,V}$	[-]	see table C2							
Concrete pry-out failure										
Pry-out Factor	k_8	[-]	2,0							
Concrete edge failure										
Effective length of anchor	l_f	[mm]	min(h_{ef}; 12 d_{nom})						min (h_{ef}; 300mm)	
Outside diameter of anchor	d_{nom}	[mm]	8	10	12	16	20	24	27	30
Installation factor	γ_{inst}	[-]	1,0							

Table C6: Characteristic values for **threaded rods** under **tension load, seismic action**, performance category **C1**

Threaded rod					**M8**	**M10**	**M12**	**M16**	**M20**	**M24**	**M27**	**M30**
Steel failure												
Characteristic resistance			$N_{Rk,s,C1}$	[kN]	$1,0 \cdot N_{Rk,s}$							
Partial factor			$\gamma_{Ms,V}$	[-]	see Table C1							
Combined pull-out and concrete failure												
Characteristic bond resistance in concrete C20/25 to C50/60												
Temperature range	I: 40°C/24°C	dry or wet concrete	$\tau_{Rk,C1}$	[N/mm²]	2,5	3,1	3,7	3,7	3,7	3,8	4,5	4,5
	II: 80°C/50°C				1,6	2,2	2,7	2,7	2,7	2,8	3,1	3,1
	III: 120°C/72°C				1,3	1,6	2,0	2,0	2,0	2,1	2,4	2,4
	I: 40°C/24°C	waterfilled drill hole	$\tau_{Rk,C1}$	[N/mm²]	2,5	2,5	3,7	3,7	no performance assessed			
	II: 80°C/50°C				1,6	1,9	2,7	2,7				
	III: 120°C/72°C				1,3	1,6	2,0	2,0				
Installation factor												
Dry or wet concrete			γ_{inst}	[-]	1,0	1,2						
Waterfilled drill hole			γ_{inst}	[-]	1,4				no performance assessed			

Table C7: Characteristic values for **threaded rods** under **shear load, seismic action,** performance category **C1**

Threaded rod				**M8**	**M10**	**M12**	**M16**	**M20**	**M24**	**M27**	**M30**
Steel failure											
Characteristic resistance		$V_{Rk,s,C1}$	[kN]	$0,7 \cdot V^0_{Rk,s}$							
Partia factor		$\gamma_{Ms,V}$	[-]	See Table C2							
Factor for annular gap											
Factor for anchorages	without hole clearance	α_{gap}	[-]	1,0							
	with hole clearance between fastener and fixture	α_{gap}	[-]	0,5							

Table C8: Characteristic values of **tension loads** for **internally threaded anchor rods**

Internally threaded anchor rod					**IG-M 6**	**IG-M 8**	**IG-M 10**	**IG-M 12**	**IG-M 16**	**IG-M 20**
Steel failure [1]										
Characteristic resistance, steel zinc plated, strength class		5.8	$N_{Rk,s}$	[kN]	10	17	29	42	76	123
		8.8	$N_{Rk,s}$	[kN]	16	27	46	67	121	196
Partial factor			$\gamma_{Ms,N}$	[-]	1,5					
Characteristic resistance, stainless steel A4 / HCR, strength class 70			$N_{Rk,s}$	[kN]	14	26	41	59	110	124 [2]
Partial factor			$\gamma_{Ms,N}$	[-]	1,87					2,86
Combined pull-out and concrete cone failure										
Characteristic bond resistance in uncracked concrete C20/25										
Temperature range	I: 40°C/24°C	dry and wet concrete	$\tau_{Rk,ucr}$	[N/mm²]	12	12	12	12	11	9,0
	II: 80°C/50°C			[N/mm²]	9,0	9,0	9,0	9,0	8,5	6,5
	III: 120°C/72°C			[N/mm²]	6,5	6,5	6,5	6,5	6,5	5,0
	I: 40°C/24°C	waterfilled drill hole	$\tau_{Rk,ucr}$	[N/mm²]	8,5	8,5	8,5	no performance assessed		
	II: 80°C/50°C			[N/mm²]	6,5	6,5	6,5			
	III: 120°C/72°C			[N/mm²]	5,0	5,0	5,0			
Characteristic bond resistance in cracked concrete C20/25										
Temperature range	I: 40°C/24°C	dry and wet concrete	$\tau_{Rk,cr}$	[N/mm²]	5,0	5,5	5,5	5,5	5,5	6,5
	II: 80°C/50°C			[N/mm²]	3,5	4,0	4,0	4,0	4,0	4,5
	III: 120°C/72°C			[N/mm²]	2,5	3,0	3,0	3,0	3,0	3,5
	I: 40°C/24°C	waterfilled drill hole	$\tau_{Rk,cr}$	[N/mm²]	4,0	5,5	5,5	no performance assessed		
	II: 80°C/50°C			[N/mm²]	3,0	4,0	4,0			
	III: 120°C/72°C			[N/mm²]	2,5	3,0	3,0			
Reductionfactor ψ^0_{sus} in concrete C20/25										
Temperature range	I: 40°C/24°C	dry and wet concrete waterfilled drill hole	ψ^0_{sus}	[-]	0,73					
	II: 80°C/50°C				0,65					
	III: 120°C/72°C				0,57					
Increasing factors for τ_{Rk}			ψ_c	C25/30	1,02					
				C30/37	1,04					
				C35/45	1,07					
				C40/50	1,08					
				C45/55	1,09					
				C50/60	1,10					
Concrete cone failure and splitting failure										
Relevant parameter					see Table C3					
Installation factor										
dry and wet concrete			γ_{inst}	[-]	1,2					
waterfilled drill hole			γ_{inst}	[-]	1,4			no performance determined		

[1] fastening screws or threaded rods (incl. nut and washer) must comply with the appropriate material and property class of the internally threaded anchor rod. The characteristic tension resistance for steel failure of the given strength class are valid for the internally threaded anchor rod and the fastening element.

[2] for VMU-IG M20: Internally threaded rod: strength class 50; Fastening screws or threaded rods (incl. nut and washer): strength class 70

Table C9: Characteristic values for **internally threaded anchor rods** under **shear loads**

Internally threaded anchor rod				**IG-M 6**	**IG-M 8**	**IG-M 10**	**IG-M 12**	**IG-M 16**	**IG-M 20**
Steel failure without lever arm[1]									
Characteristic resistance, steel zinc plated, strength class	5.8	$V^0_{Rk,s}$	[kN]	6	10	17	25	45	74
	8.8	$V^0_{Rk,s}$	[kN]	8	14	23	34	60	98
Partial factor		$\gamma_{Ms,V}$	[-]	1,25					
Characteristic resistance, stainless steel A4 / HCR, strength class	70	$V^0_{Rk,s}$	[kN]	7	13	20	30	55	62 [2]
Partial factor		$\gamma_{Ms,V}$	[-]	1,56					2,38
Ductility factor		k_7	[-]	1,0					
Steel failure with lever arm[1]									
Characteristic bending moment, steel zinc plated, strength class	5.8	$M^0_{Rk,s}$	[Nm]	8	19	37	66	167	325
	8.8	$M^0_{Rk,s}$	[Nm]	12	30	60	105	267	519
Partial factor		$\gamma_{Ms,V}$	[-]	1,25					
Characteristic bending resistance, stainless steel A4 / HCR, strength class	70	$M^0_{Rk,s}$	[Nm]	11	26	53	92	234	643 [2]
Partial factor		$\gamma_{Ms,V}$	[-]	1,56					2,38
Concrete pry-out failure									
Pry-out factor		k_8	[-]	2,0					
Concrete edge failure									
Effective length of anchor		l_f	[mm]	min(h_{ef}; 12 d_{nom})					min (h_{ef}; 300mm)
Outside diameter of anchor		d_{nom}	[mm]	10	12	16	20	24	30
Installation factor		γ_{inst}	[-]	1,0					

[1] fastening screws or threaded rods (incl. nut and washer) must comply with the appropriate material and property class of the internally threaded anchor rod. The characteristic shear resistance for steel failure of the given strength class are valid for the internally threaded anchor rod and the fastening element

[2] for VMU-IG M20: Internally threaded rod: strength class 50; Fastening screws or threaded rods (incl. nut and washer): strength class 70

Table C10: Characteristic values for **rebar** under **tension loads**

Rebar					**Ø 8**	**Ø 10**	**Ø 12**	**Ø 14**	**Ø 16**	**Ø 20**	**Ø 25**	**Ø 28**	**Ø 32**
Steel failure													
Characteristic resistance			$N_{Rk,s}$	[kN]	$A_s \cdot f_{uk}$ 1)								
Cross sectional area			A_s	[mm²]	50	79	113	154	201	314	491	616	804
Partial factor			$\gamma_{Ms,N}$	[-]	1,4 2)								
Combined pull-out and concrete cone failure													
Characteristic bond resistance in uncracked concrete C20/25													
Temperature range	I: 40°C/24°C	dry and wet concrete	$\tau_{Rk,ucr}$	[N/mm²]	10	12	12	12	12	12	11	10	8,5
	II: 80°C/50°C				7,5	9,0	9,0	9,0	9,0	9,0	8,0	7,0	6,0
	III: 120°C/72°C				5,5	6,5	6,5	6,5	6,5	6,5	6,0	5,0	4,5
	I: 40°C/24°C	waterfilled drill hole	$\tau_{Rk,ucr}$	[N/mm²]	7,5	8,5	8,5	8,5	8,5	no performance assessed			
	II: 80°C/50°C				5,5	6,5	6,5	6,5	6,5				
	III: 120°C/72°C				4,0	5,0	5,0	5,0	5,0				
Characteristic bond resistance in cracked concrete C20/25													
Temperature range	I: 40°C/24°C	dry and wet concrete	$\tau_{Rk,cr}$	[N/mm²]	4,0	5,0	5,5	5,5	5,5	5,5	5,5	6,5	6,5
	II: 80°C/50°C				2,5	3,5	4,0	4,0	4,0	4,0	4,0	4,5	4,5
	III: 120°C/72°C				2,0	2,5	3,0	3,0	3,0	3,0	3,0	3,5	3,5
	I: 40°C/24°C	waterfilled drill hole	$\tau_{Rk,cr}$	[N/mm²]	4,0	4,0	5,5	5,5	5,5	no performance assessed			
	II: 80°C/50°C				2,5	3,0	4,0	4,0	4,0				
	III: 120°C/72°C				2,0	2,5	3,0	3,0	3,0				
Reductionfactor ψ^0_{sus} in concrete C20/25													
Temperature range	I: 40°C/24°C	dry and wet concrete waterfilled drill hole	ψ^0_{sus}	[-]	0,73								
	II: 80°C/50°C				0,65								
	III: 120°C/72°C				0,57								
Increasing factors for τ_{Rk}			ψ_c	C25/30	1,02								
				C30/37	1,04								
				C35/45	1,07								
				C40/50	1,08								
				C45/55	1,09								
				C50/60	1,10								
Concrete cone failure and splitting failure													
Relevant parameter					see Table C3								
Installation factor													
dry and wet concrete			γ_{inst}	[-]	1,0	1,2							
waterfilled drill hole			γ_{inst}	[-]	1,4					no performance assessed			

1) f_{uk} shall be taken from the specifications of reinforcing bars
2) in absence of national regulation

Table C11: Characteristic values for **rebar** under **shear load**

Rebar			**Ø 8**	**Ø 10**	**Ø 12**	**Ø 14**	**Ø 16**	**Ø 20**	**Ø 25**	**Ø 28**	**Ø 32**
Steel failure without lever arm											
Characteristic resistance	$V^0_{Rk,s}$	[kN]	$0{,}50 \cdot A_s \cdot f_{uk}$ [1)]								
Cross sectional area	A_s	[mm²]	50	79	113	154	201	314	491	616	804
Partial factor	$\gamma_{Ms,V}$	[-]	1,5 [2)]								
Ductility factor	k_7	[-]	1,0								
Steel failure with lever arm											
Characteristic bending moment	$M^0_{Rk,s}$	[Nm]	$1{,}2 \cdot W_{el} \cdot f_{uk}$ [1)]								
Elastic section modulus	W_{el}	[mm³]	50	98	170	269	402	785	1534	2155	3217
Partial factor	$\gamma_{Ms,V}$	[-]	1,5 [2)]								
Concrete pry-out failure											
Factor	k_8	[-]	2,0								
Concrete edge failure											
Effective length of anchor	l_f	[mm]	min(h_{ef}; 12 d_{nom})						min(h_{ef}; 300mm)		
Outside diameter of anchor	d_{nom}	[mm]	8	10	12	14	16	20	25	28	32
Installation factor	γ_{inst}	[-]	1,0								

1) f_{uk} shall be taken from the specifications of reinforcing bars
2) in absence of national regulation

Table C12: Characteristic values for **rebar** under **seismic action, tension load** performance category **C1**

Rebar					**Ø 8**	**Ø 10**	**Ø 12**	**Ø 14**	**Ø 16**	**Ø 20**	**Ø 25**	**Ø 28**	**Ø 32**
Steel failure													
Characteristic resistance			$N_{Rk,s,C1}$	[kN]	$A_s \cdot f_{uk}$ [1]								
Cross sectional area			A_s	[mm²]	50	79	113	154	201	314	491	616	804
Partial factor			$\gamma_{Ms,N}$	[-]	1,4 [2]								
Combined pull-out and concrete cone failure													
Characteristic bond resistance in concrete C20/25 to C50/60													
Temperature range	I: 40°C/24°C	dry and wet concrete	$\tau_{Rk,C1}$	[N/mm²]	2,5	3,1	3,7	3,7	3,7	3,7	3,8	4,5	4,5
	II: 80°C/50°C				1,6	2,2	2,7	2,7	2,7	2,7	2,8	3,1	3,1
	III: 120°C/72°C				1,3	1,6	2,0	2,0	2,0	2,0	2,1	2,4	2,4
	I: 40°C/24°C	waterfilled drill hole	$\tau_{Rk,C1}$	[N/mm²]	2,5	2,5	3,7	3,7	3,7	no performance assessed			
	II: 80°C/50°C				1,6	1,9	2,7	2,7	2,7				
	III: 120°C/72°C				1,3	1,6	2,0	2,0	2,0				
Installation factor													
dry and wet concrete			γ_{inst}	[-]	1,0	1,2							
waterfilled drill hole			γ_{inst}	[-]	1,4					no performance assessed			

[1] f_{uk} shall be taken from the specifications of reinforcing bars
[2] in absence of national regulation

Table C13: Characteristic values for **rebar** under **seismic action, shear load,** performance category **C1**

Rebar			**Ø 8**	**Ø 10**	**Ø 12**	**Ø 14**	**Ø 16**	**Ø 20**	**Ø 25**	**Ø 28**	**Ø 32**
Steel failure without lever arm											
Characteristic resistance	$V_{Rk,s,C1}$	[kN]	$0,35 \cdot A_s \cdot f_{uk}$ [1]								
Cross sectional area	A_s	[mm²]	50	79	113	154	201	314	491	616	804
Partial factor	$\gamma_{Ms,V}$	[-]	1,5 [2]								
Ductility factor	k_7	[-]	1,0								

[1] f_{uk} shall be taken from the specifications of reinforcing bars
[2] in absence of national regulation

Table C14: **Displacement factor** under **tension loads**[1)]
(threaded rod and internally threaded anchor rod)

Threaded rod			M8	M10 IG-M6	M12 IG-M8	M16 IG- M10	M20 IG-M12	M24 IG-M16	M27	M30 IG-M20
Uncracked concrete C20/25, static and quasi-static action										
Temperature range I: 40°C/24°C	δ_{N0}-factor	$[\frac{mm}{N/mm^2}]$	0,021	0,023	0,026	0,031	0,036	0,041	0,045	0,049
	$\delta_{N\infty}$-factor		0,030	0,033	0,037	0,045	0,052	0,060	0,065	0,071
Temperature range II: 80°C/50°C	δ_{N0}-factor		0,050	0,056	0,063	0,075	0,088	0,100	0,110	0,119
	$\delta_{N\infty}$-factor		0,072	0,081	0,090	0,108	0,127	0,145	0,159	0,172
Temperature range III: 120°C/72°C	δ_{N0}-factor		0,050	0,056	0,063	0,075	0,088	0,100	0,110	0,119
	$\delta_{N\infty}$-factor		0,072	0,081	0,090	0,108	0,127	0,145	0,159	0,172
Cracked concrete C20/25, static and quasi-static action										
Temperature range I: 40°C/24°C	δ_{N0}-factor	$[\frac{mm}{N/mm^2}]$	0,090		0,070					
	$\delta_{N\infty}$-factor		0,105		0,105					
Temperature range II: 80°C/50°C	δ_{N0}-factor		0,219		0,170					
	$\delta_{N\infty}$-factor		0,255		0,245					
Temperature range III: 120°C/72°C	δ_{N0}-factor		0,219		0,170					
	$\delta_{N\infty}$-factor		0,255		0,245					

1) Calculation of the displacement
$\delta_{N0} = \delta_{N0}$- factor $\cdot \tau$; τ: acting bond stress for tension load
$\delta_{N\infty} = \delta_{N\infty}$-factor $\cdot \tau$;

Table C15: **Displacement factor** under **shear load**[1)]
(threaded rod and internally threaded anchor rod)

Threaded rod			M8	M10 IG-M6	M12 IG-M8	M16 IG- M10	M20 IG-M12	M24 IG-M16	M27	M30 IG-M20
Uncracked concrete C20/25, static and quasi-static action										
All temperature ranges	δ_{V0}-factor	$[\frac{mm}{N/mm^2}]$	0,06	0,06	0,05	0,04	0,04	0,03	0,03	0,03
	$\delta_{V\infty}$-factor		0,09	0,08	0,08	0,06	0,06	0,05	0,05	0,05
Cracked concrete C20/25, static and quasi-static action										
All temperature ranges	δ_{V0}-factor	$[\frac{mm}{N/mm^2}]$	0,12	0,12	0,11	0,10	0,09	0,08	0,08	0,07
	$\delta_{V\infty}$-factor		0,18	0,18	0,17	0,15	0,14	0,13	0,12	0,10

1) Calculation of the displacement
$\delta_{V0} = \delta_{V0}$-factor $\cdot$ V; V: acting shear load
$\delta_{V\infty} = \delta_{V\infty}$-factor $\cdot$ V;

Table C16: **Displacement factor** under **tension load**[1] (rebar)

Rebar			**∅ 8**	**∅ 10**	**∅ 12**	**∅ 14**	**∅ 16**	**∅ 20**	**∅ 25**	**∅ 28**	**∅ 32**
Uncracked concrete C20/25, static and quasi-static action											
Temperature range I: 40°C/24°C	δ_{N0}-factor	$[\frac{mm}{N/mm^2}]$	0,021	0,023	0,026	0,028	0,031	0,036	0,043	0,047	0,052
	$\delta_{N\infty}$-factor		0,030	0,033	0,037	0,041	0,045	0,052	0,061	0,071	0,075
Temperature range II: 80°C/50°C	δ_{N0}-factor		0,050	0,056	0,063	0,069	0,075	0,088	0,104	0,113	0,126
	$\delta_{N\infty}$-factor		0,072	0,081	0,090	0,099	0,108	0,127	0,149	0,163	0,181
Temperature range III: 120°C/72°C	δ_{N0}-factor		0,050	0,056	0,063	0,069	0,075	0,088	0,104	0,113	0,126
	$\delta_{N\infty}$-factor		0,072	0,081	0,090	0,099	0,108	0,127	0,149	0,163	0,181
Cracked concrete C20/25, static and quasi-static action											
Temperature range I: 40°C/24°C	δ_{N0}-factor	$[\frac{mm}{N/mm^2}]$	0,090	0,070							
	$\delta_{N\infty}$-factor		0,105	0,105							
Temperature range II: 80°C/50°C	δ_{N0}-factor		0,219	0,170							
	$\delta_{N\infty}$-factor		0,255	0,245							
Temperature range III: 120°C/72°C	δ_{N0}-factor		0,219	0,170							
	$\delta_{N\infty}$-factor		0,255	0,245							

[1] Calculation of the displacement
$\delta_{N0} = \delta_{N0}\text{-factor} \cdot \tau$; τ: acting bond stress for tension load
$\delta_{N\infty} = \delta_{N\infty}\text{-factor} \cdot \tau$;

Table C17: **Displacement factor** under **shear load**[1] (rebar)

Rebar			**∅ 8**	**∅ 10**	**∅ 12**	**∅ 14**	**∅ 16**	**∅ 20**	**∅ 25**	**∅ 28**	**∅ 32**
Uncracked concrete C20/25, static and quasi-static action											
All temperature ranges	δ_{V0}-factor	$[\frac{mm}{N/mm^2}]$	0,06	0,05	0,05	0,04	0,04	0,04	0,03	0,03	0,03
	$\delta_{V\infty}$-factor		0,09	0,08	0,08	0,06	0,06	0,05	0,05	0,04	0,04
Cracked concrete C20/25, static and quasi-static action											
All temperature ranges	δ_{V0}-factor	$[\frac{mm}{N/mm^2}]$	0,12	0,12	0,11	0,11	0,10	0,09	0,08	0,07	0,06
	$\delta_{V\infty}$-factor		0,18	0,18	0,17	0,16	0,15	0,14	0,12	0,11	0,10

[1] Calculation of the displacement
$\delta_{V0} = \delta_{V0}\text{-factor} \cdot V$; V: acting shear load
$\delta_{V\infty} = \delta_{V\infty}\text{-factor} \cdot V$;